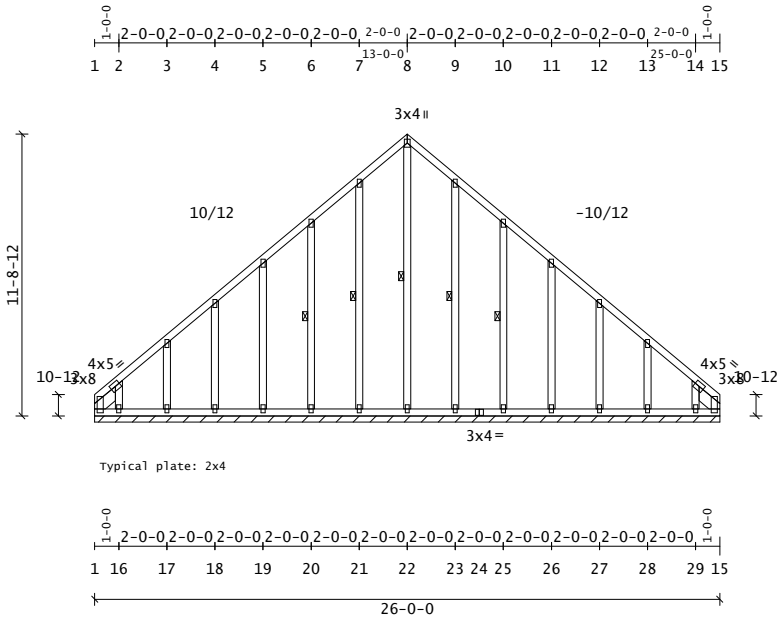


Customer: GREEN-R-PANEL

SID:
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Truss Mfr. Contact:



Truss Weight = 168.8 lb

Building Code: NBCC2015/TPIC2014
Building Category: Part4 - Gyp ceiling
Importance Category: Normal
Loading(psf) Factor Kh applied: Yes
TCSL(S) 30.7 Dur Fac Kd = 1.00 (Snow)
TCLL 0.0 Kd = 1.00 (Live)
TCDL 10.0 Kd = 1.15 (Wind)
BCLL 10.0
BCDL 10.0
Spacing: 2-00-00 oc Plies: 1

-----Snow Load Specs-----
Ground Snow Ss = 43.9 psf
Rain Load Sr = 4.2 psf
Importance Factor Is = 1.00
Basic roof snow load factor Cb = 0.80
Wind exposure Cw = 1.00
Slope factor Cs = From Geometry
Slippery roof factor = No
Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II

-----Wind Load Specs-----
Design Method = MWFRS
Input pressure = 10.2 psf
Exposure Category = Open
Building Category = Category 2
End Zone = No
Apply to left end vertical = Yes
Apply to right end vertical = Yes

-----Additional Design Checks-----
2000 lb TC Safe Load: No
Unbalanced Snow Loads: Yes

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2
Slider	2x6	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	6	-	7	0.08
Max CSI in BC PANEL	1	-	16	0.06
Max CSI in Web	22	-	8	0.25

...Mem...	Ten	Comp	.CSI.
TC 1- 8	241	115	0.08
8-15	241	115	0.08
BC 1-24	295	64	0.06
15-24	295	64	0.06
Web 1- 2	130	607	0.03
2-16	435	179	0.06
3-17	138	284	0.04
4-18	128	272	0.07
5-19	129	274	0.15
6-20	133	270	0.08
7-21	135	303	0.15
8-22	0	352	0.25
9-23	135	303	0.15
10-25	133	270	0.08
11-26	129	274	0.15
12-27	128	272	0.07
13-28	138	284	0.04
14-15	130	607	0.03
14-29	435	179	0.06

Maximum Factored Reaction Summary

-----Reaction Summary(Lbs)-----						
Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	04-00	624	115	26-00-00		
16	1-00-00	249	431	26-00-00		
29	25-00-00	249	431	26-00-00		
15	25-08-00	624	115	26-00-00		
Max Horiz = -336 / +336 at Joint 22						
Reactions not shown: down < 400 and up < 150						
---- Reaction Summary (plf) ----						
Jnt-Jnt	React	-Up-	--Width-			
1- 15	94	0	26-00-00	(reduced)		

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
	Pinned (Wall)	1596	519	-372	1039

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
The maximum rake overhang length is 12.0".
Gable requires 7/16" OSB sheathing on front from 0-00-00 to 26-00-00; connection details to be provided by the Building Designer.
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE.
This truss is not symmetric - proper orientation is critical.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.00)	16-17
Vert DL	L/360	L/999(-0.00)	16-17
Vert TL	L/180	L/999(-0.00)	16-17
Horz TL	1.00in	(0.01)	@Jt15

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single: 20- 6 21- 7 22- 8 23- 9 25-10

Plate offsets (X, Y):

(None unless indicated below)

Plate Info

Plate	Grip	psi	Shear	pli	Tens.Pli
AS	Max	Min	@0	@45	@90
20G	341	249	753	637	589 1144 1130

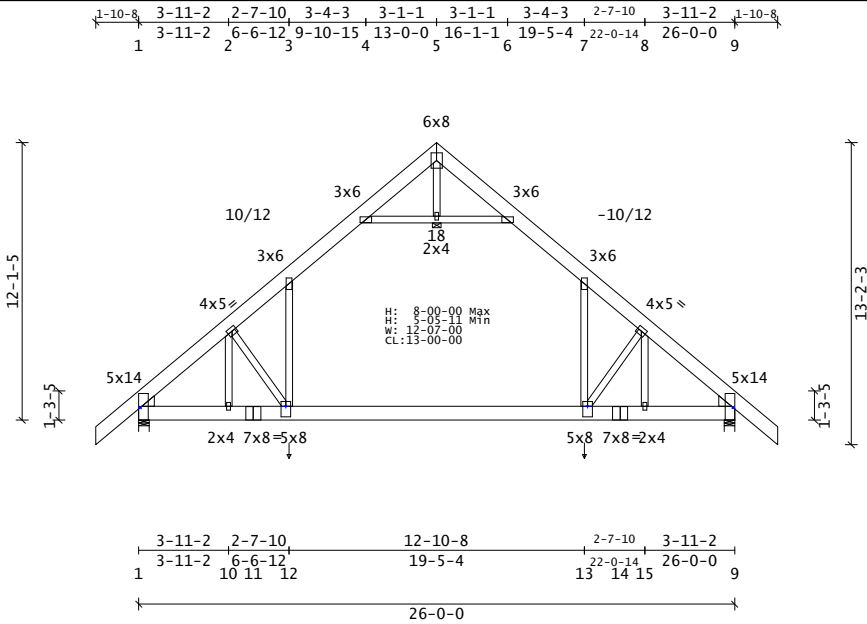
Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with CCMC 13326-L & 13418-L. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

Customer: GREEN-R-PANEL

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Truss Mfr. Contact:



Truss Weight = 226.5 lb

Building Code: NBCC2015/TPIC2014
Building Category: Part4 - Gyp ceiling
Importance Category: Normal
Loading(psf) Factor Kh applied: Yes
TCSL(S) 30.7 Dur Fac Kd = 1.00 (Snow)
TCLL 0.0 Kd = 1.00 (Live)
TCDL 10.0 Kd = 1.15 (Wind)
BCLL 10.0
BCDL 10.0
Spacing: 2-00-00 oc Plies: 1

-----Snow Load Specs-----
Ground Snow Ss = 43.9 psf
Rain Load Sr = 4.2 psf
Importance Factor Is = 1.00
Basic roof snow load factor Cb = 0.80
Wind exposure Cw = 1.00
Slope factor Cs = From Geometry
Slippery roof factor = No
Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II

-----Wind Load Specs-----
Design Method = MWFRS
Input pressure = 10.2 psf
Exposure Category = Open
Building Category = Category 2
End Zone = No
Apply to left end vertical = Yes
Apply to right end vertical = Yes

-----Additional Design Checks-----
2000 lb TC Safe Load: No
Unbalanced Snow Loads: Yes
Attic Floor:
LL = 40.0 psf, DL = 10.0 psf
Attic Wall: DL = 5.0 psf
Attic Ceiling: DL = 5.0 psf

Material Summary

TC	2x8	SPF	1950/1.7
BC	2x8	SPF	#2
	2x8	SPF	2400/2.0 11-14
Webs	2x4	SPF	#2
Wedge	2x6	SPF	#2
TB	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	2 - 3	0.64
Max CSI in BC PANEL	10 - 11	0.56
Max CSI in Web	4 - 18	0.67

...Mem...	Ten	Comp	.CSI.
TC OH- 1	170	0	0.06
1- 2	0	3584	0.19
2- 3	0	3977	0.64
3- 4	0	2420	0.63
4- 5	428	272	0.50
5- 6	428	272	0.50
6- 7	0	2420	0.63
7- 8	0	3977	0.64
8- 9	0	3584	0.19
9-OH	170	0	0.06
BC 1-10	2538	0	0.44
9-15	2538	0	0.44
10-11	2538	0	0.56
11-12	2538	0	0.52
12-13	2523	0	0.53
13-14	2538	0	0.52
14-15	2538	0	0.56
Web 2-10	93	1113	0.18
2-12	466	588	0.12
3-12	2319	0	0.37
4-18	0	2962	0.67
5-18	66	0	0.01
6-18	0	2962	0.67
7-13	2319	0	0.37
8-13	471	590	0.12
8-15	94	1113	0.18

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	0	3160	0	05-08 04-02	SPF	
9	26-00-00	3160	0	05-08 04-02	SPF	
Max Horiz = -381 / +381 at Joint 1						

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
1	Pinned (Wall)	913	775	-169	812
9	H Roll (Wall)	913	775	-169	812

Loads Summary

Attic space centred at 13-00-00 is loaded with 40.0 psf Live & 10.0 psf Dead Floor, 5.0 psf Dead Wall, 5.0 psf Dead Ceiling loads, and meets deflection criteria L/360.

See Loadcase Report for load combinations and additional details.

Unfactored Concentrated Loads (Max/Min)

Mbr	Dead	Snow	Live	Wind	Location	Dir	Desc
Web	54/54	0/0	0/0	0/0	6-06-12	Vert	
SidewallDL							
Web	54/54	0/0	0/0	0/0	19-05-04	Vert	
SidewallDL							

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/658(-0.46)	12-13
Vert DL	L/360	L/999(-0.22)	12-13
Vert TL	L/180	L/447(-0.67)	12-13
Horz TL	1.00in	(0.03) @Jt 9	
Ohng TL	OL/120	OL/875(0.05)	1- 1
Ohng TL	OL/120	OL/875(0.05)	9- 9

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Attic tie beam (TB) & walls; bracing indicated or rigid sheathing.

-----Purlins-----				
	---oc---	--From--	---To---	#Bays
TB	3-04-00	9-08-14	16-03-02	2
----- Web Bracing -- CLR -----				
Single:	4-18	18- 6		

Plate offsets (X, Y):

(None unless indicated below)
Jnt12(0,-01-08), Jnt13(0,-01-08),
Jnt1(01-11,00-07), Jnt9(-01-11,00-07)

Plate Info

Plate	Grip psi	Shear pli	Tens.Pli
AS	Max	Min	@0 @45 @90 @0 @90
20G	341	249	753 637 589 1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with CCMC 13326-L & 13418-L. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "# 18", which indicates a high tension 18 gauge plate.

Customer: GREEN-R-PANEL

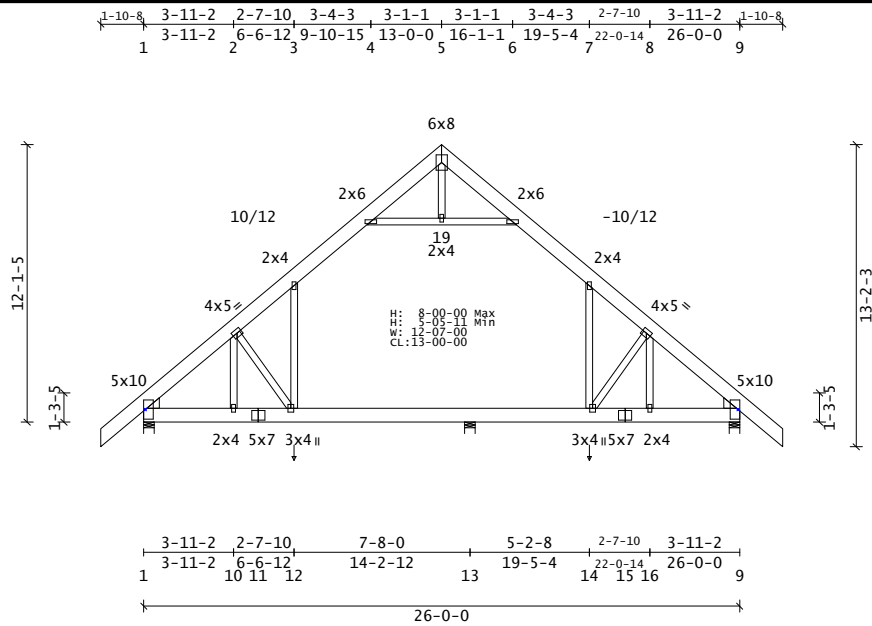
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Truss Mfr. Contact:

Plate Grip psi Shear pli Tens.Pli
JSI Metal = (INPUT = 1.00)

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Truss Weight = 206.6 lb

Building Code: NBCC2015/TPIC2014 Building Category: Part 4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 30.7 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCBL 10.0 Kd = 1.15 (Wind) BCCL 10.0 BCDL 10.0 Spacing: 2-00-00 oc Plies: 1	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes Attic Floor: LL = 40.0 psf, DL = 10.0 psf Attic Wall: DL = 5.0 psf Attic Ceiling: DL = 5.0 psf
---	---	---	---

Maximum Factored Reaction Summary

Maximum Factored Reaction Summary						
-----Reaction Summary (Lbs)-----						
Jnt	--X-Loc-	React	Up-	--Width-	-Regd	-Mat
1	0	2472	0	05-08	03-03	SPF
13	14-02-12	1946	0	05-08	02-02	SPF
9	26-00-00	2306	0	05-08	03-00	SPF
Max Horiz =		-381 /	+381	at Joint	1	

TrussSpan	Limit	Actual (in)	Location
Vert LL	L/360	L/999 (-0.26)	12-13
Vert DL	L/360	L/999 (-0.09)	12-13
Vert TL	L/180	L/859 (-0.35)	12-13
Horz TL	1.00in	(0.04)	@Jt 9
Ohng TL	OL/120	OL/694 (0.06)	1- 1
Ohng TL	OL/120	OL/977 (-0.05)	9- 9

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
1	Pinned (Wall)	871	390	-131	620
13	H Roll (Wall)	142	850	-12	423
9	H Roll (Wall)	848	309	-162	580

Loads Summary

Attic space centred at 13-00-00 is loaded with 40.0 psf Live & 10.0 psf Dead Floor, 5.0 psf Dead Wall, 5.0 psf Dead Ceiling loads, and meets deflection criteria L/360.

See Loadcase Report for load combinations and additional details.

Unfactored Concentrated Loads (Max/Min)				Wind	Location	Dir	Desc
Mbr	Dead	Snow	Live				
Web	54/54	0/0	0/0	0/0	6-06-12	Vert	
SidewallDL							
Web	54/54	0/0	0/0	0/0	19-05-04	Vert	
SidewallDL							

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Lumber and plating have been applied symmetrically.

Bracing Data Summary

```
-----Bracing Data-----
Chords; continuous except where shown
Attic tie beam (TB) & walls; bracing
indicated or rigid sheathing.
Web Bracing -- None
```

Plate offsets (X, Y):

(None unless indicated below)
Jnt1(01-11,0), Jnt9(-01-11,0)

Plate Info

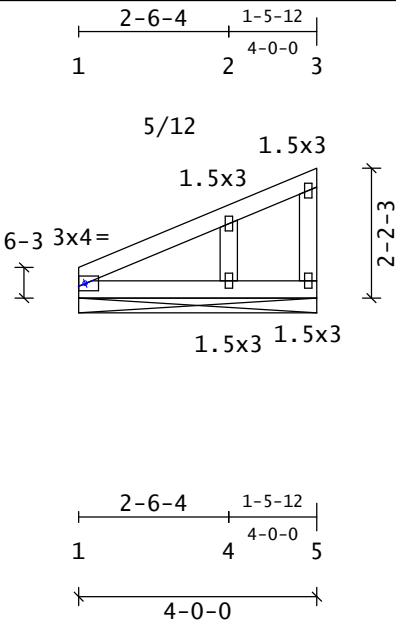
Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

...Mem...		Ten	Comp	CSI.
TC	OH- 1	170	0	0.13
	1- 2	0	2656	0.21
	2- 3	0	2383	0.59
	3- 4	96	1668	0.55
	4- 5	203	342	0.33
	5- 6	107	328	0.50
	6- 7	89	1736	0.50
	7- 8	0	2207	0.29
	8- 9	0	2454	0.20
	9-OH	170	0	0.13
BC	1-10	1867	0	0.34
	9-16	1712	0	0.30
	10-11	1867	0	0.43
	11-12	1867	0	0.63
	12-13	1520	0	0.90
	13-14	1520	0	0.90
	14-15	1712	0	0.60
Web	15-16	1712	0	0.39
	2-10	291	191	0.04
	2-12	353	892	0.20
	3-12	1050	0	0.17
	4-19	125	1515	0.94
	5-19	64	0	0.01
	6-19	125	1515	0.94
	7-14	611	80	0.10
	8-14	446	678	0.15
	8-16	336	354	0.05

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Truss Studio V
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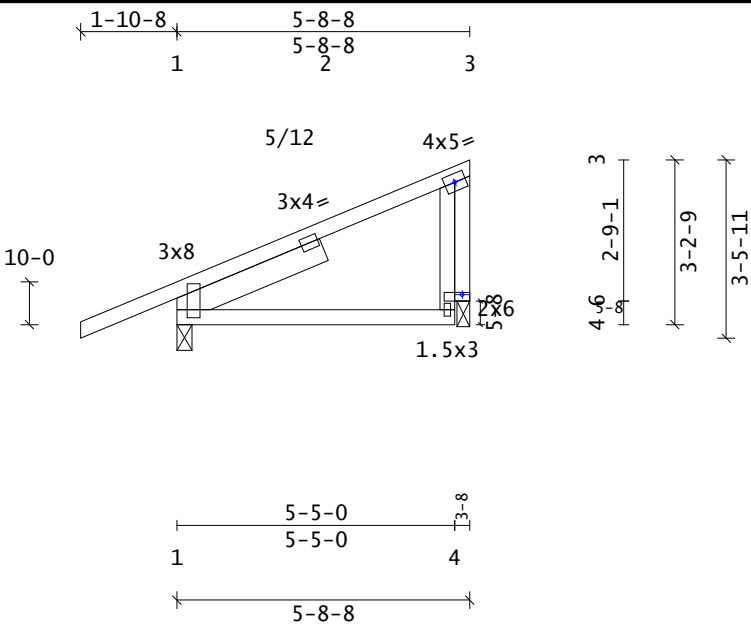


Truss includes non-inventory materials.

Truss Weight = 13.9 lb

Building Code: NBCC2015/TPIC2014	-----Snow Load Specs-----	-----Wind Load Specs-----	-----Additional Design Checks-----
Building Category: Part4 - Gyp ceiling	Ground Snow Ss = 43.9 psf	Design Method = MWFRS	2000 lb TC Safe Load: No
Importance Category: Normal	Rain Load Sr = 4.2 psf	Input pressure = 10.2 psf	Unbalanced Snow Loads: Yes
Loading(psf) Factor Kh applied: Yes	Importance Factor Is = 1.00	Exposure Category = Open	
TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow)	Basic roof snow load factor Cb = 0.80	Building Category = Category 2	
TCLL 0.0 Kd = 1.00 (Live)	Wind exposure Cw = 1.00	End Zone = No	
TCDL 10.0 Kd = 1.15 (Wind)	Slope factor Cs = From Geometry	Apply to left end vertical = Yes	
BCLL 10.0	Slippery roof factor = No	Apply to right end vertical = Yes	
BCDL 10.0	Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II		
Spacing: 2-00-00 oc Plies: 1			

Material Summary TC 2x4 SPF #2 BC 2x4 SPF #2 Webs 2x4 SPF #2	Maximum Factored Reaction Summary -----Reaction Summary(Lbs)----- Jnt --X-Loc- React -Up- --Width- -Reqd -Mat 4 2-06-04 553 61 4-00-00 Max Horiz = 0 / +93 at Joint 4 Reactions not shown: down < 400 and up < 150 ---- Reaction Summary (plf) ---- Jnt-Jnt React -Up- --Width- 1- 5 76 0 4-00-00 (reduced)	Deflection Summary TrussSpan Limit Actual(in) Location Vert LL L/360 L/999(-0.00) 1- 4 Vert DL L/360 L/999(-0.00) 1- 4 Vert TL L/180 L/999(-0.00) 1- 4 Horz TL 1.00in (0.00) @Jt 1 Bracing Data Summary -----Bracing Data----- Chords; continuous except where shown Web Bracing -- None
Member Forces Summary Max CSI in TC PANEL 1 - 2 0.12 Max CSI in BC PANEL 1 - 4 0.06 Max CSI in Web 4 - 2 0.04 ...Mem... Ten Comp .CSI. TC 1- 3 102 106 0.12 BC 1- 5 84 0 0.06 Web 2- 4 95 409 0.04 3- 5 15 70 0.02	Unfactored Reaction Summary Jnt Type Snow Live Wind Dead Pinned (Beam) 384 80 -90 160 Loads Summary See Loadcase Report for load combinations and additional details. Notes Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents. The maximum rake overhang length is 12.0". Gable requires 7/16" OSB sheathing on front from 0-00-00 to 4-00-00; connection details to be provided by the Building Designer. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Plate marked as unavailable in catalogue: 2 - 1.5x3 Plate marked as unavailable in catalogue: 3 - 1.5x3 Plate marked as unavailable in catalogue: 4 - 1.5x3 Plate marked as unavailable in catalogue: 5 - 1.5x3	Plate offsets (X, Y): (None unless indicated below) Jnt1(00-11,0) Plate Info Plate Grip psi Shear pli Tens.Pli AS Max Min @0 @45 @90 @0 @90 20G 341 249 753 637 589 1144 1130 Plate Placement Tol. = 0.250 inches Plate Rotation Tol. = 5.0 Deg. JSI Grip = (INPUT = 0.90) JSI Metal = (INPUT = 1.00)



Truss includes non-inventory materials.

Truss Weight = 30.3 lb

Building Code: NBCC2015/TPIC2014 Building Category: Part4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCDL 10.0 Kd = 1.15 (Wind) BCLL 10.0 BCDL 10.0 Spacing: 2-00-00 oc Plies: 1	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes
--	---	---	--

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2
Slider	2x6	SPF	#2
BB	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1	-	1	0.43
Max CSI in BC PANEL	1	-	4	0.24
Max CSI in Web	3	-	3	0.20

Maximum Factored Reaction Summary

-----Reaction Summary(Lbs)-----						
Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	0	979	22	03-08	01-08	SPF
6	5-07-00	557	54	03-00	01-08	SPF
Max Horiz = 0 / +151 at Joint 1						
Max Horiz = 0 / +151 at Joint 6						
TC Bearing @ 3 net reaction = +508						

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
1	Pinned (Beam)	479	55	-68	164
6	H Roll (Beam)	250	52	-66	103

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Plate marked as unavailable in catalogue: 4 - 1.5x3
Vertical Step - Ensure adequate drift loads are applied
Undefined TC Bearing End Condition - Engineering Verification Required

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.04)	1- 4
Vert DL	L/360	L/999(-0.03)	1- 4
Vert TL	L/180	L/806(-0.08)	1- 4
Horz TL	1.00in	(0.03)	@Jt 1
Ohng TL	OL/120	OL/732(0.06)	1- 1

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

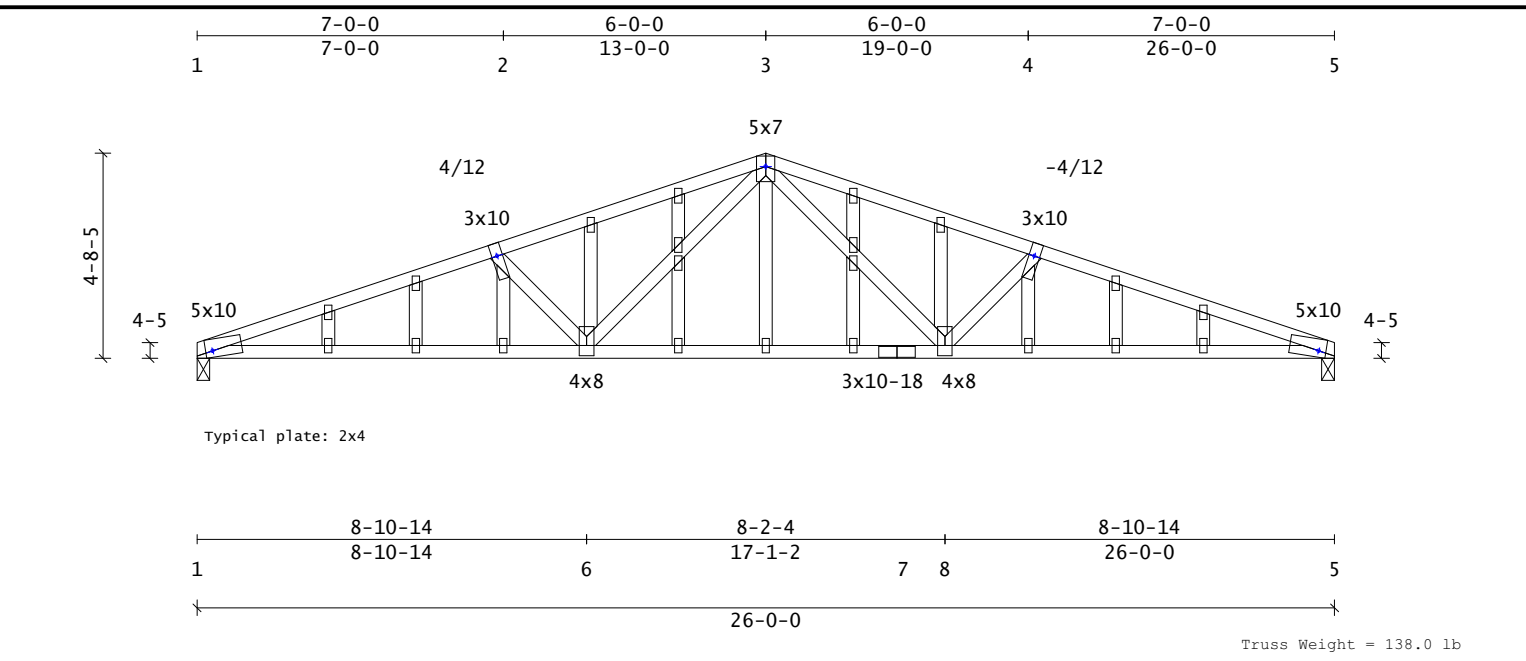
Plate offsets (X, Y):

(None unless indicated below)
Jnt3(00-01,0), Jnt6(-01-04,-00-08)

Plate Info

Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)



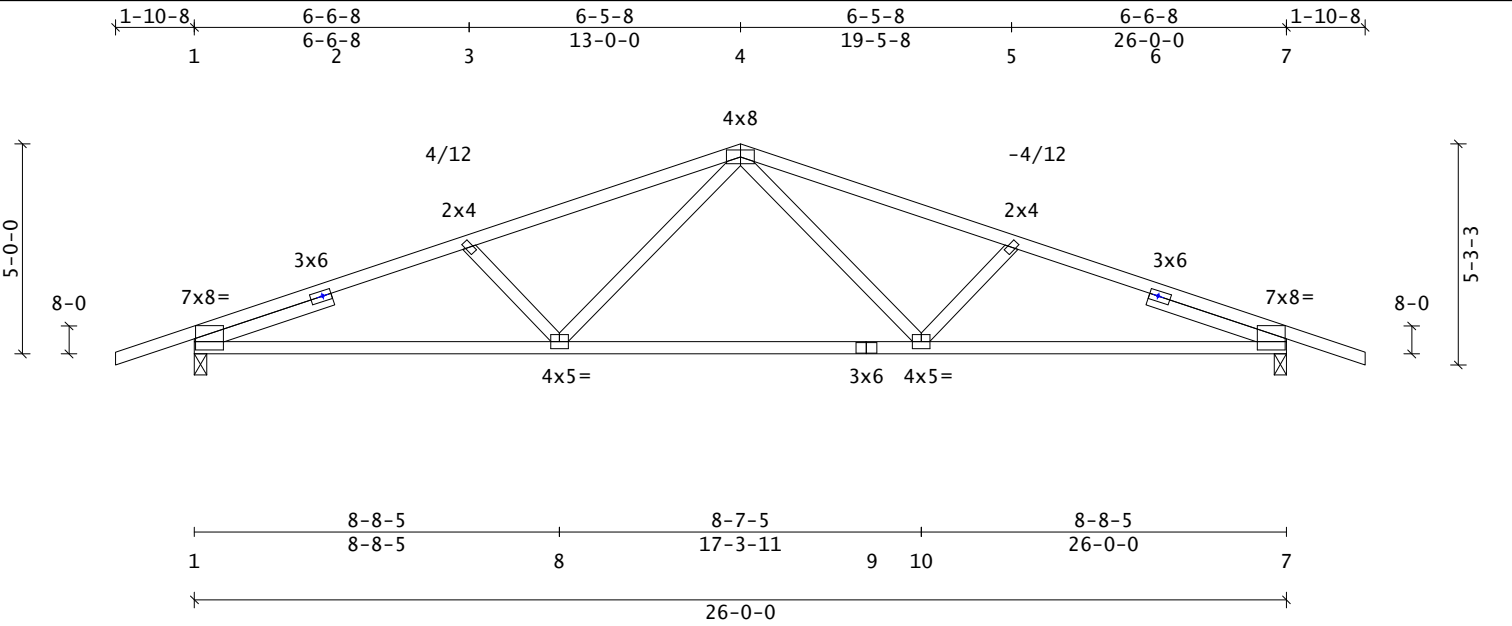
Building Code: NBCC2015/TPIC2014 Building Category: Part4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCDL 10.0 Kd = 1.15 (Wind) BCLL 10.0 BCDL 10.0 Spacing: 2'-00-00 oc Plies: 1	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.17 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes
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Material Summary TC 2x4 SPF 1650/1.5 BC 2x4 SPF 2100/1.8 Webs 2x4 SPF #2	Maximum Factored Reaction Summary -----Reaction Summary(Lbs)----- Jnt --X-Loc- React -Up- --Width- -Regd -Mat 1 0 2443 0 03-08 02-05 SPF 5 26'-00-00 2443 0 03-08 02-05 SPF Max Horiz = -69 / +69 at Joint 1	Deflection Summary TrussSpan Limit Actual(in) Location Vert LL L/360 L/999(-0.27) 8- 5 Vert DL L/360 L/999(-0.20) 8- 5 Vert TL L/180 L/656(-0.46) 8- 5 Horz TL 1.00in (0.10) @Jt 5
Member Forces Summary Max CSI in TC PANEL 1 - 2 0.84 Max CSI in BC PANEL 1 - 6 0.68 Max CSI in Web 6 - 3 0.29	Unfactored Reaction Summary Jnt Type Snow Live Wind Dead 1 Pinned (Beam) 1022 259 -350 519 5 H Roll (Beam) 1022 260 -350 520	Bracing Data Summary -----Bracing Data----- Chords; continuous except where shown Web Bracing -- None
Notes Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents. The maximum rake overhang length is 12.0". Gable requires 7/16" OSB sheathing on front from 0'-00-00 to 26'-00-01; connection details to be provided by the Building Designer. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. The upper top chord (UTC) may be notched 1.5" deep x 3.5" wide at 24"o.c. max. for outlookers. Do not notch in the heel areas marked or anywhere there is a single chord member. Do not cut the connector plates. This truss is not symmetric - proper orientation is critical.	Loads Summary See Loadcase Report for load combinations and additional details.	Plate offsets (X, Y): (None unless indicated below) Jnt1(02-14,01-03), Jnt2(00-08,-01-07), Jnt3(0,-00-10), Jnt4(-00-08,-01-07), Jnt5(-02-14,01-03)
Plate Info Plate Grip psi Shear pli Tens.Pli AS Max Min @0 @45 @90 @0 @90 20G 341 249 753 637 589 1144 1130 HS18G 344 220 1103 890 979 1829 1808	Plate Placement Tol. = 0.250 inches Plate Rotation Tol. = 5.0 Deg. JSI Grip = (INPUT = 0.90) JSI Metal = (INPUT = 1.00)	

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Truss Mfr. Contact:



Building Code: NBCC2015/TPIC2014 Building Category: Part4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCDL 10.0 Kd = 1.15 (Wind) BCLL 10.0 BCDL 10.0 Spacing: 2'-00-00 oc Plies: 1	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.17 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes
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Material Summary

TC	2x4	SPF	1650/1.5
BC	2x4	SPF	1650/1.5
Webs	2x4	SPF	#2
Slider	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	3	-	4	0.89
Max CSI in BC PANEL	1	-	8	0.87
Max CSI in Web	1	-	2	0.39

	Mem...	Ten	Comp	.CSI.
TC	OH- 1	98	0	0.34
	1- 2	0	2864	0.27
	2- 3	56	5145	0.53
	3- 4	0	4650	0.89
	4- 5	0	4650	0.89
	5- 6	56	5145	0.53
	6- 7	0	2864	0.27
BC	7-OH	98	0	0.34
	1- 8	4825	0	0.87
	7-10	4825	0	0.87
	8- 9	3327	0	0.65
Web	9-10	3327	0	0.65
	1- 2	260	2674	0.39
	3- 8	284	1011	0.16
	4- 8	1527	79	0.24
	4-10	1527	80	0.24
	5-10	284	1011	0.16
	6- 7	260	2674	0.39

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	0	2720	45	03-08	02-15	SPF
7	26-00-00	2720	45	03-08	02-15	SPF
Max Horiz	=	-83 /	+83	at Joint	1	

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
1	Pinned (Beam)	1169	259	-365	564
7	H Roll (Beam)	1169	259	-365	564

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. This truss is not symmetric - proper orientation is critical.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.30)	8-10
Vert DL	L/360	L/999(-0.20)	8-10
Vert TL	L/180	L/611(-0.50)	8-10
Horz TL	1.00in	(0.11)	@Jt 7
Ohng TL	OL/120	OL/861(-0.05)	1- 1
Ohng TL	OL/120	OL/856(-0.05)	7- 7

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

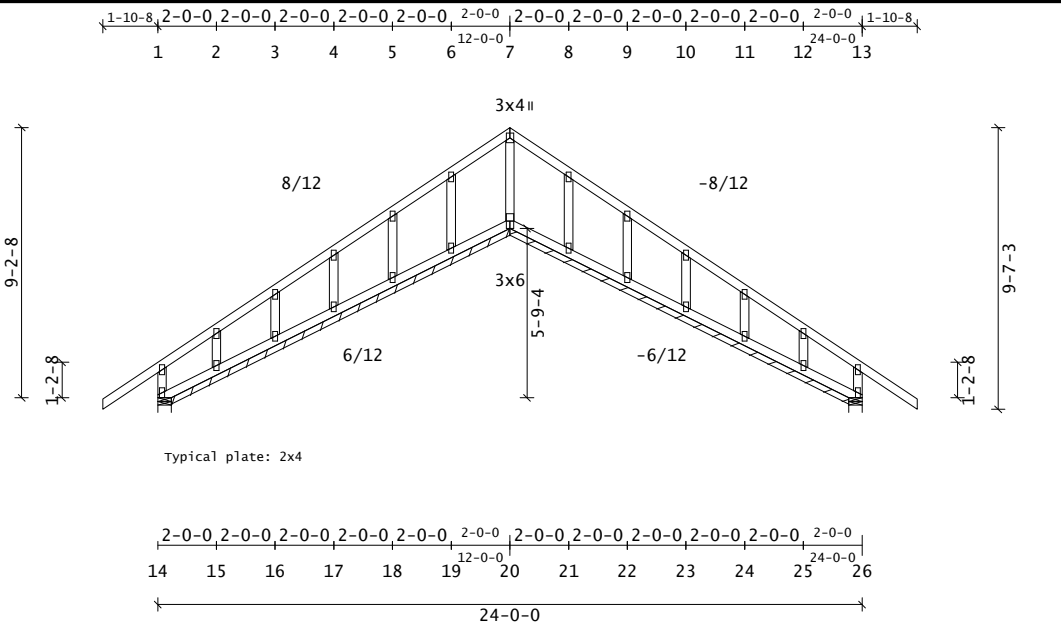
(None unless indicated below)
Jnt2(-00-08,-00-03), Jnt6(00-08,-00-03)

Plate Info

Plate	Grip psi		Shear pli			Tens.Pli	
AS	Max	Min	@0	@45	@90	@0	@90
20G	341	249	753	637	589	1144	1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with CCMC 13326-L & 13418-L. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Truss Weight = 104.7 lb

Building Code: NBCC2015/TPIC2014 Building Category: Part4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 36.1 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCDL 10.0 Kd = 1.15 (Wind) BCLL 10.0 BCDL 10.0 Spacing: 2-00-00 oc Plies: 1	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes
--	---	---	--

Material Summary				
TC	2x4	SPF	#2	
BC	2x4	SPF	#2	
Webs	2x4	SPF	#2	

Member Forces Summary				
Max CSI in TC PANEL	1	-	1	0.41
Max CSI in BC PANEL	25	-	26	0.12
Max CSI in Web	14	-	1	0.14

...	Mem...	Ten	Comp	.CSI.
TC	1-7	168	0	0.41
	7-13	168	0	0.41
BC	14-20	181	181	0.11
	20-26	143	143	0.12
Web	1-14	86	504	0.14
	2-15	138	264	0.02
	3-16	99	332	0.03
	4-17	106	312	0.03
	5-18	104	306	0.03
	6-19	115	353	0.04
	7-20	0	454	0.06
	8-21	115	353	0.04
	9-22	104	306	0.03
	10-23	106	312	0.03
	11-24	101	332	0.03
	12-25	132	264	0.02
	13-26	70	504	0.14

Maximum Factored Reaction Summary						
Jnt	--X-Loc-	React	-Up-	--Width-	-Regd	-Mat
14	0	593	170	05-08	01-08	SPF
26	24-00-00	593	48	05-08	01-08	SPF

16	4-00-00	427	57	11-06-08		
17	6-00-00	402	72	11-06-08		
19	10-00-00	456	75	11-06-08		
20	12-00-00	565	0	11-06-08		
21	14-00-00	456	75	11-06-08		
23	18-00-00	402	71	11-06-08		
24	20-00-00	427	60	11-06-08		
Max Horiz = -275 / +275 at Joint 14						
Reactions not shown: down < 400 and up < 150						
---- Reaction Summary (plf) ----						
Jnt-Jnt	React	-Up-	--Width-			
14- 20	30	0	11-06-08	(reduced)		
20- 26	59	16	11-06-08	(reduced)		

Unfactored Reaction Summary					
Jnt	Type	Snow	Live	Wind	Dead
14	Pinned (Wall)	306	18	112	92
	H Roll (Wall)	919	237	234	466
	H Roll (Wall)	841	206	-10	399
26	H Roll (Wall)	306	18	-4	92

Loads Summary
See Loadcase Report for load combinations and additional details.

Notes
Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
The maximum rake overhang length is 12.0".
Gable requires 7/16" OSB sheathing on front from 0-00-00 to 24-00-00; connection details to be provided by the Building Designer.
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Bearing @ 6-02-12 exceeds wall width. Bearing enhancement may be required.
Bearing @ 17-09-04 exceeds wall width. Bearing enhancement may be required.
Lumber and plating have been applied symmetrically.

Deflection Summary				
TrussSpan	Limit	Actual(in)	Location	
Vert LL	L/360	L/999(0.00)	14-15	
Vert DL	L/360	L/999(-0.00)	19-20	
Vert TL	L/180	L/999(-0.00)	14-15	
Horz TL	1.00in	(0.00)	@Jt26	
Ohng TL	OL/120	OL/548(-0.08)	1- 1	
Ohng TL	OL/120	OL/558(-0.08)	13-13	

Bracing Data Summary
-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):
(None unless indicated below)

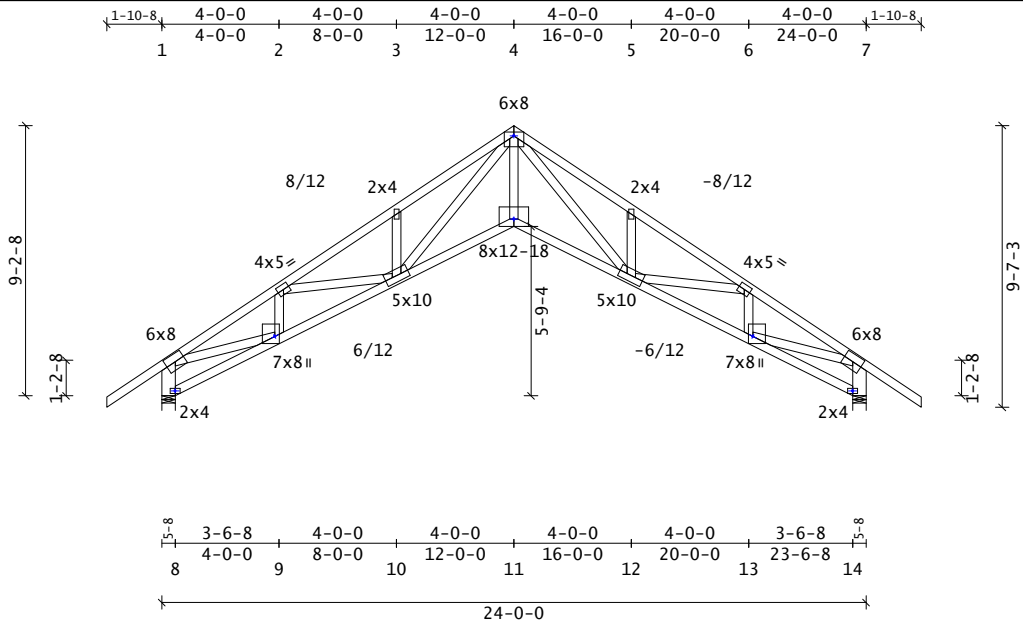
Plate Info							
Plate	Grip	psi	Shear	pli	Tens.	Pli	
AS	Max	Min	@0	@45	@90	@0	@90
20G	341	249	753	637	589	1144	1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Weight = 147.8 lb

Building Code: NBCC2015/TPIC2014
Building Category: Part4 - Gyp ceiling
Importance Category: Normal
Loading(psf) Factor Kh applied: Yes
TCSL(S) 36.1 Dur Pac Kd = 1.00 (Snow)
TCLL 0.0 Kd = 1.00 (Live)
TCDL 10.0 Kd = 1.15 (Wind)
BCLL 10.0
BCDL 10.0
Spacing: 2-00-00 oc Plies: 1

-----Snow Load Specs-----
Ground Snow Ss = 43.9 psf
Rain Load Sr = 4.2 psf
Importance Factor Is = 1.00
Basic roof snow load factor Cb = 0.80
Wind exposure Cw = 1.00
Slope factor Cs = From Geometry
Slippery roof factor = No
Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II

-----Wind Load Specs-----
Design Method = MWFRS
Input pressure = 10.2 psf
Exposure Category = Open
Building Category = Category 2
End Zone = No
Apply to left end vertical = Yes
Apply to right end vertical = Yes

-----Additional Design Checks-----
2000 lb TC Safe Load: No
Unbalanced Snow Loads: Yes

Material Summary

TC	2x4	SPF	#2	
BC	2x4	SPF	#2	
Webs	2x4	SPF	#2	
	2x6	SPF	#2	8-1 14-7

Member Forces Summary

Max CSI in TC PANEL	3	-	4	0.69
Max CSI in BC PANEL	10	-	11	0.85
Max CSI in Web	11	-	4	0.69

...	Mem...	Ten	Comp	CSI.
TC	OH- 1	168	0	0.41
	1- 2	19	5043	0.58
	2- 3	0	5842	0.60
	3- 4	105	5869	0.69
	4- 5	105	5869	0.69
	5- 6	0	5842	0.60
	6- 7	19	5043	0.58
BC	7-OH	168	0	0.41
	8- 9	338	329	0.12
	9-10	4606	66	0.84
	10-11	4648	0	0.85
	11-12	4648	0	0.85
Web	12-13	4606	66	0.84
	13-14	338	329	0.12
	1- 8	98	2309	0.15
	1- 9	4184	0	0.67
	2- 9	93	945	0.10
	2-10	719	0	0.11
	3-10	234	707	0.08
	4-10	1490	665	0.31
	4-11	4294	0	0.69
	4-12	1490	665	0.31
	5-12	234	707	0.08
	6-12	719	0	0.11
	6-13	93	945	0.10
	7-13	4184	0	0.67
	7-14	98	2309	0.15

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
8	0	2398	0	05-08	02-04	SPF
14	24-00-00	2398	0	05-08	02-04	SPF
Max Horiz	=	-278	/	+278	at Joint	8
Max Horiz	=	-278	/	+278	at Joint	14

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
8	Pinned (Wall)	1001	239	-144	524
14	H Roll (Wall)	1001	239	-144	524

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/788(-0.35)	10-11
Vert DL	L/360	L/999(-0.19)	10-11
Vert TL	L/180	L/509(-0.54)	10-11
Horz TL	1.00in	(0.63)	@Jt14
Ohng TL	OL/120	OL/230(0.20)	1- 1
Ohng TL	OL/120	OL/230(0.20)	7- 7

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)
Jnt4(0,-01-07), Jnt8(0,00-01),
Jnt9(-01-10,01-00), Jnt11(0,00-15),
Jnt13(01-10,01-00), Jnt14(0,00-01)

Plate Info

Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130
HS18G	344	220	1103	890	979	1829 1808

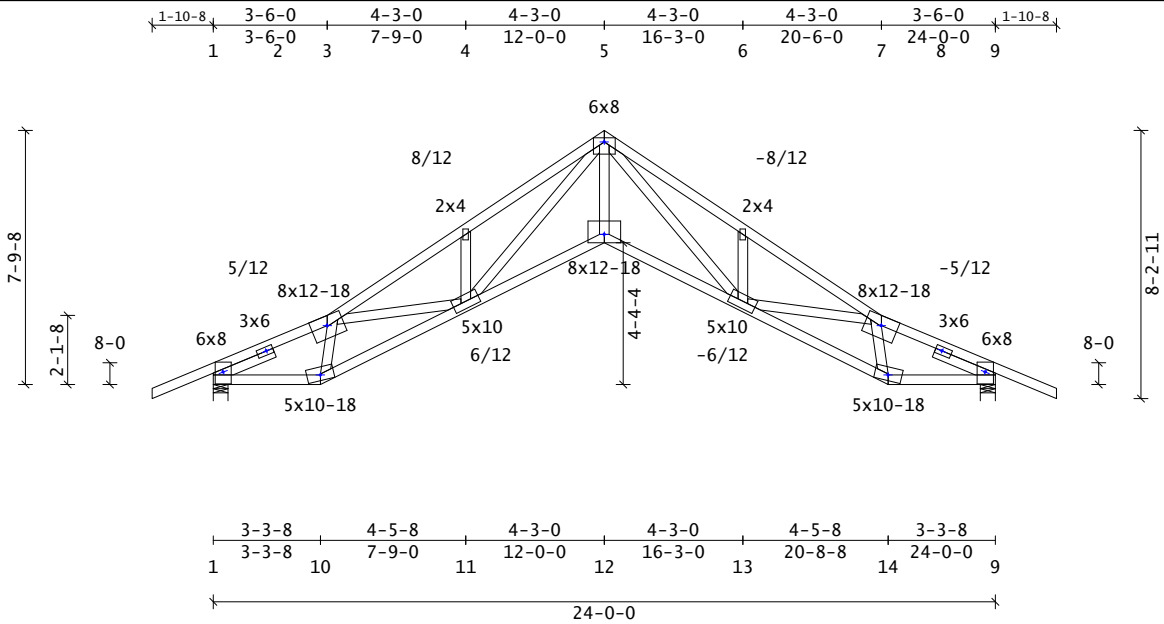
Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with CCMC 13326-L & 13418-L. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

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Building Code: NBCC2015/TPIC2014 Building Category: Part4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 36.1 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCDL 10.0 Kd = 1.15 (Wind) BCDL 10.0 BCDL 10.0 Spacing: 2-00-00 oc Plies: 1		-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes
--	--	---	---	--

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2
Slider	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	4 - 5	0.77
Max CSI in BC PANEL	1 - 10	0.86
Max CSI in Web	12 - 5	0.69

...	Mem...	Ten	Comp	.CSI.
TC	OH-	1	125	0 0.43
	1- 2	0	2027	0.49
	2- 3	0	3967	0.61
	3- 4	0	5897	0.70
	4- 5	95	5961	0.77
	5- 6	0	5961	0.77
	6- 7	0	5897	0.70
	7- 8	0	3967	0.61
	8- 9	0	2027	0.49
	9-OH	125	0	0.43
BC	1-10	3549	0	0.86
	9-14	3549	0	0.86
	10-11	4250	0	0.80
	11-12	4671	0	0.86
	12-13	4671	0	0.86
	13-14	4250	0	0.80
Web	1- 2	6	2306	0.24
	3-10	60	1796	0.19
	3-11	1091	0	0.17
	4-11	278	809	0.09
	5-11	1560	569	0.30
	5-12	4318	0	0.69
	5-13	1560	664	0.35
	6-13	278	809	0.09
	7-13	1091	0	0.17
	7-14	0	1796	0.19
	8- 9	3	2306	0.24

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	0	2450	0	05-08	03-06	SPF
9	24-00-00	2450	0	05-08	03-06	SPF
Max Horiz = -163 / +163 at Joint 1						

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
1	Pinned (Wall)	1036	239	-245	524
9	H Roll (Wall)	1036	240	-245	525

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/811(-0.34)	11-12
Vert DL	L/360	L/999(-0.19)	11-12
Vert TL	L/180	L/521(-0.53)	11-12
Horz TL	1.00in	(0.38)	@Jt 9
Ohng TL	OL/120	OL/700(0.06)	1- 1
Ohng TL	OL/120	OL/700(0.06)	9- 9

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)
Jnt2(-00-07,-00-03), Jnt3(00-03,-00-07),
Jnt5(0,-01-07), Jnt7(-00-03,-00-07),
Jnt8(00-07,-00-03), Jnt10(-00-02,00-07),
Jnt12(0,00-15), Jnt14(00-02,00-07),
Jnt1(0,-00-05), Jnt9(0,-00-05)

Plate Info

Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130
HS18G	344	220	1103	890	979	1829 1808

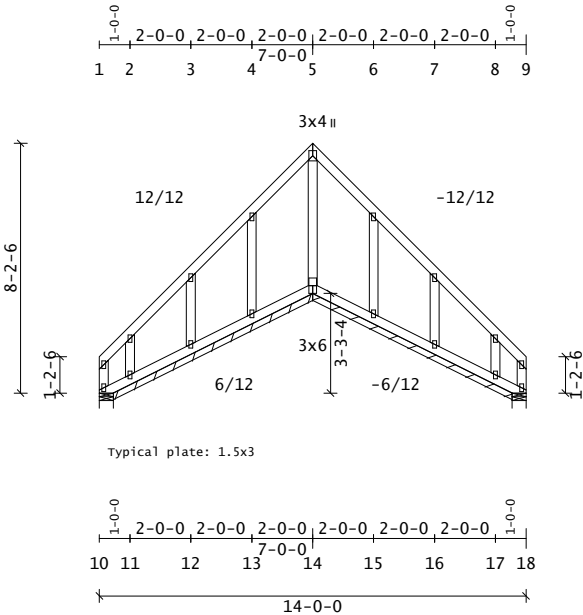
Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Mfr. Contact:



Truss includes non-inventory materials.

Truss Weight = 65.5 lb

Building Code: NBCC2015/TPIC2014
Building Category: Part4 - Gyp ceiling
Importance Category: Normal
Loading(psf) Factor Kh applied: Yes
TCSL(S) 26.2 Dur Fac Kd = 1.00 (Snow)
TCCLL 0.0 Kd = 1.00 (Live)
TCDL 10.0 Kd = 1.15 (Wind)
BCCLL 10.0
BCDL 10.0
Spacing: 2-00-00 oc Plies: 1

-----Snow Load Specs-----
Ground Snow Ss = 43.9 psf
Rain Load Sr = 4.2 psf
Importance Factor Is = 1.00
Basic roof snow load factor Cb = 0.80
Wind exposure Cw = 1.00
Slope factor Cs = From Geometry
Slippery roof factor = No
Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II

-----Wind Load Specs-----
Design Method = MWFRS
Input pressure = 10.2 psf
Exposure Category = Open
Building Category = Category 2
End Zone = No
Apply to left end vertical = Yes
Apply to right end vertical = Yes

-----Additional Design Checks-----
2000 lb TC Safe Load: No
Unbalanced Snow Loads: Yes

Material Summary

TC 2x4 SPF #2
BC 2x4 SPF #2
Webs 2x4 SPF #2

Member Forces Summary

Max CSI in TC PANEL 1 - 2 0.08
Max CSI in BC PANEL 10 - 11 0.09
Max CSI in Web 14 - 5 0.10

...	Mem...	Ten	Comp	.CSI.
TC	1- 5	195	249	0.08
	5- 9	195	249	0.08
BC	10-14	167	125	0.09
	14-18	167	125	0.09
Web	1-10	138	213	0.09
	2-11	174	214	0.02
	3-12	147	262	0.03
	4-13	157	290	0.05
	5-14	0	437	0.10
	6-15	157	290	0.05
	7-16	147	262	0.03
	8-17	174	214	0.02
	9-18	138	213	0.09

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
10	0	337	246	05-08	01-08	SPF
18	14-00-00	337	246	05-08	01-08	SPF
11	1-00-00	288	207	6-06-08		
14	7-00-00	559	0	6-06-08		
17	13-00-00	288	207	6-06-08		
Max Horiz = -213 / +213 at Joint 10						
Max Horiz = -213 / +213 at Joint 18						
Reactions not shown: down < 400 and up < 150						
---- Reaction Summary (plf) ----						
Jnt-Jnt	React	-Up-	--Width-			
10- 14	58	0	6-06-08	(reduced)		
14- 18	113	34	6-06-08	(reduced)		

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
10	Pinned (Wall)	20	5	222	16
	H Roll (Wall)	420	149	257	357
	H Roll (Wall)	373	118	33	284
18	H Roll (Wall)	20	5	222	16

Loads Summary

See Loadcase Report for load combinations and additional details.
Dead loads have been slope adjusted for any pitch 12.00/12 or greater

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
The maximum rake overhang length is 12.0".
Gable requires 7/16" OSB sheathing on front from 0-00-00 to 14-00-00; connection details to be provided by the Building Designer.
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Plate marked as unavailable in catalogue: 1 - 1.5x3
Plate marked as unavailable in catalogue: 2 - 1.5x3
Plate marked as unavailable in catalogue: 3 - 1.5x3
Plate marked as unavailable in catalogue: 4 - 1.5x3
Plate marked as unavailable in catalogue: 6 - 1.5x3
Plate marked as unavailable in catalogue: 7 - 1.5x3
Plate marked as unavailable in catalogue: 8 - 1.5x3
Plate marked as unavailable in catalogue: 9 - 1.5x3
Plate marked as unavailable in catalogue: 10 - 1.5x3

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.00)	13-14
Vert DL	L/360	L/999(-0.00)	13-14
Vert TL	L/180	L/999(-0.00)	13-14
Horz TL	1.00in	(0.00) @Jt18	

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)

Plate Info

Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with CCMC 13326-L & 13418-L. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

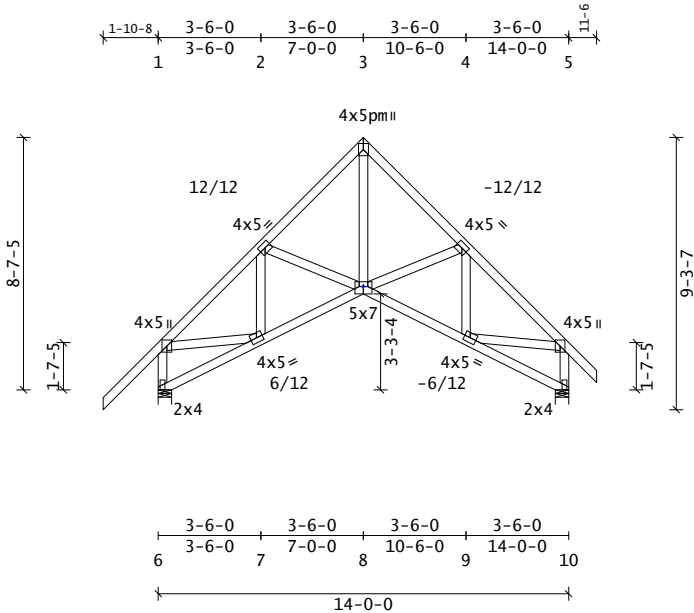
D:\SST_Riverbend\Clie... Component Solutions
Truss Studio V
2024.3.2.1

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Truss Mfr. Contact:

Plate marked as unavailable in catalogue: 11 - 1.5x3
Plate marked as unavailable in catalogue: 12 - 1.5x3
Plate marked as unavailable in catalogue: 13 - 1.5x3
Plate marked as unavailable in catalogue: 15 - 1.5x3
Plate marked as unavailable in catalogue: 16 - 1.5x3
Plate marked as unavailable in catalogue: 17 - 1.5x3
Plate marked as unavailable in catalogue: 18 - 1.5x3
Bearing @ 3-08-12 exceeds wall width. Bearing enhancement may be required.
Bearing @ 10-03-04 exceeds wall width. Bearing enhancement may be required.
Lumber and plating have been applied symmetrically.



Truss Weight = 83.8 lb

Building Code: NBCC2015/TPIC2014 Building Category: Part4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 26.2 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCDL 10.0 Kd = 1.15 (Wind) BCLL 10.0 BCDL 10.0 Spacing: 2-00-00 oc Plies: 1	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes
--	---	---	--

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1	-	1	0.35
Max CSI in BC PANEL	8	-	9	0.24
Max CSI in Web	8	-	3	0.20

	Mem...	Ten	Comp	.CSI.
TC	OH- 1	179	0	0.35
	1- 2	0	1468	0.27
	2- 3	29	1263	0.19
	3- 4	43	1266	0.19
	4- 5	0	1472	0.19
BC	5-OH	90	0	0.09
	6- 7	293	302	0.11
	7- 8	1089	78	0.23
	8- 9	1102	83	0.24
	9-10	312	283	0.11
Web	1- 6	22	1282	0.13
	1- 7	952	0	0.15
	2- 7	63	209	0.03
	2- 8	246	233	0.04
	3- 8	1233	0	0.20
	4- 8	242	274	0.05
	4- 9	73	215	0.03
	5- 9	964	0	0.15
	5-10	0	1149	0.12

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
6	0	1345	0	05-08	02-11	SPF
10	14-00-00	1213	0	05-08	02-07	SPF
Max Horiz	=	-239	/	+255	at Joint	6
Max Horiz	=	-239	/	+255	at Joint	10

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
6	Pinned (Wall)	469	139	-113	401
10	H Roll (Wall)	410	140	-98	365

Loads Summary

See Loadcase Report for load combinations and additional details.
Dead loads have been slope adjusted for any pitch 12.00/12 or greater

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.03)	7- 8
Vert DL	L/360	L/999(-0.02)	7- 8
Vert TL	L/180	L/999(-0.05)	7- 8
Horz TL	1.00in	(0.05)	@Jt10
Ohng TL	OL/120	OL/470(-0.10)	1- 1
Ohng TL	OL/120	OL/981(0.02)	5- 5

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)
Jnt8(0,-00-09)

Plate Info

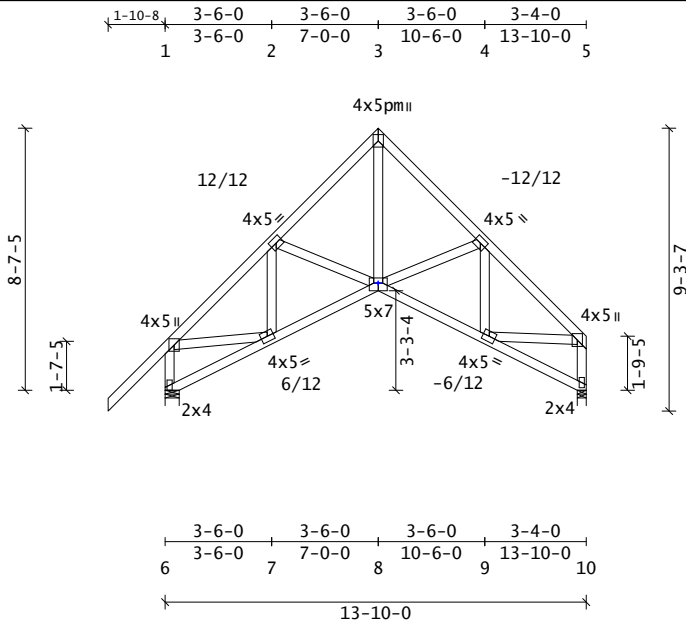
Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Mfr. Contact:



Truss Weight = 81.9 lb

Building Code: NBCC2015/TPIC2014		-----Snow Load Specs-----		-----Wind Load Specs-----		-----Additional Design Checks-----	
Building Category: Part4 - Gyp ceiling		Ground Snow Ss = 43.9 psf		Design Method = MWFRS		2000 lb TC Safe Load: No	
Importance Category: Normal		Rain Load Sr = 4.2 psf		Input pressure = 10.2 psf		Unbalanced Snow Loads: Yes	
Loading(psf) Factor Kh applied: Yes		Importance Factor Is = 1.00		Exposure Category = Open			
TCSL(S) 26.2 Dur Fac Kd = 1.00 (Snow)		Basic roof snow load factor Cb = 0.80		Building Category = Category 2			
TCLL 0.0 Kd = 1.00 (Live)		Wind exposure Cw = 1.00		End Zone = No			
TCDL 10.0 Kd = 1.15 (Wind)		Slope factor Cs = From Geometry		Apply to left end vertical = Yes			
BCLL 10.0		Slippery roof factor = No		Apply to right end vertical = Yes			
BCDL 10.0		Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II					
Spacing: 2-00-00 oc Plies: 1							

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1	-	1	0.35
Max CSI in BC PANEL	7	-	8	0.23
Max CSI in Web	8	-	3	0.19

TC	Mem.	Ten	Comp	.CSI.
	OH- 1	179	0	0.35
	1-2	4	1450	0.27
	2-3	32	1240	0.19
	3-4	63	1242	0.20
	4-5	0	1404	0.19
BC	6-7	263	301	0.11
	7-8	1075	79	0.23
	8-9	1057	78	0.23
Web	9-10	310	251	0.10
	1-6	33	1273	0.13
	1-7	939	0	0.15
	2-7	62	204	0.03
	2-8	246	236	0.04
	3-8	1199	0	0.19
	4-8	228	259	0.04
	4-9	79	244	0.03
	5-9	921	0	0.15
	5-10	0	1033	0.11

Maximum Factored Reaction Summary

-----Reaction Summary(Lbs)-----							
Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat	
6		0 1336	0	05-08	02-10	SPF	
10	13-10-00	1093	0	03-08	01-12	SPF	
Max Horiz	=	-211 /	+255	at Joint	6		
Max Horiz	=	-211 /	+255	at Joint	10		

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
6	Pinned (Wall)	466	138	-112	398
10	H Roll (Wall)	361	138	-81	329

Loads Summary

See Loadcase Report for load combinations and additional details.
Dead loads have been slope adjusted for any pitch 12.00/12 or greater

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.02)	7- 8
Vert DL	L/360	L/999(-0.02)	7- 8
Vert TL	L/180	L/999(-0.05)	7- 8
Horz TL	1.00in	(0.05) @Jt10	
Ohng TL	OL/120	OL/469(-0.10)	1- 1

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)
Jnt8(0,-00-09)

Plate Info

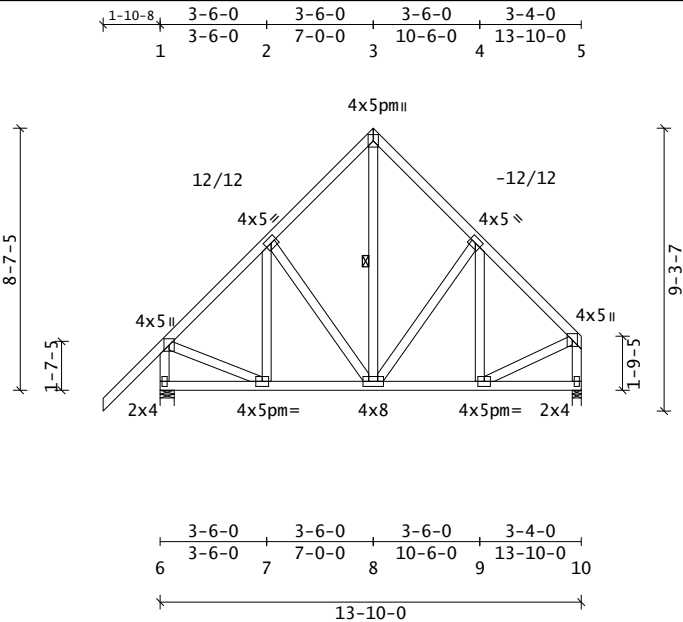
Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Mfr. Contact:



Truss Weight = 91.7 lb

Building Code: NBCC2015/TPIC2014
Building Category: Part4 - Gyp ceiling
Importance Category: Normal
Loading(psf) Factor Kh applied: Yes
TCSL(S) 26.2 Dur Fac Kd = 1.00 (Snow)
TCLL 0.0 Kd = 1.00 (Live)
TCDL 10.0 Kd = 1.15 (Wind)
BCLL 10.0
BCDL 10.0
Spacing: 2-00-00 oc Plies: 1

-----Snow Load Specs-----
Ground Snow Ss = 43.9 psf
Rain Load Sr = 4.2 psf
Importance Factor Is = 1.00
Basic roof snow load factor Cb = 0.80
Wind exposure Cw = 1.00
Slope factor Cs = From Geometry
Slippery roof factor = No
Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II

-----Wind Load Specs-----
Design Method = MWFRS
Input pressure = 10.2 psf
Exposure Category = Open
Building Category = Category 2
End Zone = No
Apply to left end vertical = Yes
Apply to right end vertical = Yes

-----Additional Design Checks-----
2000 lb TC Safe Load: No
Unbalanced Snow Loads: Yes

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1	-	1	0.35
Max CSI in BC PANEL	7	-	8	0.17
Max CSI in Web	8	-	4	0.14

Mem...		Ten	Comp	.CSI.
TC	OH- 1	179	0	0.35
	1- 2	16	1021	0.26
	2- 3	156	821	0.18
	3- 4	150	823	0.19
	4- 5	14	981	0.19
BC	6- 7	211	254	0.07
	7- 8	633	25	0.17
	8- 9	622	30	0.16
	9-10	254	211	0.07
Web	1- 6	0	1271	0.13
	1- 7	673	0	0.11
	2- 7	95	99	0.02
	2- 8	202	315	0.13
	3- 8	609	124	0.09
	4- 8	200	340	0.14
	4- 9	82	132	0.03
	5- 9	677	0	0.11
	5-10	0	1031	0.10

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
6	0	1336	0	05-08	02-10	SPF
10	13-10-00	1093	0	03-08	01-12	SPF
Max Horiz	=	-211	/	+255	at Joint	6
Max Horiz	=	-211	/	+255	at Joint	10

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
6	Pinned (Wall)	466	138	-111	398
10	H Roll (Wall)	361	138	-84	329

Loads Summary

See Loadcase Report for load combinations and additional details.
Dead loads have been slope adjusted for any pitch 12.00/12 or greater

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements.
Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.01)	7- 8
Vert DL	L/360	L/999(-0.01)	8- 9
Vert TL	L/180	L/999(-0.02)	7- 8
Horz TL	1.00in	(0.00)	@Jt10
Ohng TL	OL/120	OL/442(-0.10)	1- 1

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single: 8- 3

Plate offsets (X, Y):

(None unless indicated below)

Plate Info

Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

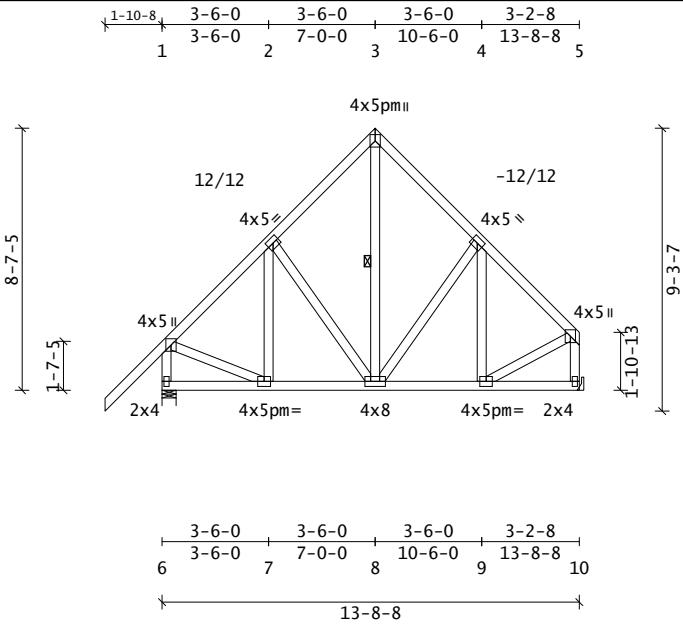
Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with CCMC 13326-L & 13418-L. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

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Truss Mfr. Contact:



Truss Weight = 91.5 lb

Building Code: NBCC2015/TPIC2014
Building Category: Part4 - Gyp ceiling
Importance Category: Normal
Loading(psf) Factor Kh applied: Yes
TCSL(S) 26.2 Dur Fac Kd = 1.00 (Snow)
TCLL 0.0 Kd = 1.00 (Live)
TCDL 10.0 Kd = 1.15 (Wind)
BCLL 10.0
BCDL 10.0
Spacing: 2-00-00 oc Plies: 1

-----Snow Load Specs-----
Ground Snow Ss = 43.9 psf
Rain Load Sr = 4.2 psf
Importance Factor Is = 1.00
Basic roof snow load factor Cb = 0.80
Wind exposure Cw = 1.00
Slope factor Cs = From Geometry
Slippery roof factor = No
Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II

-----Wind Load Specs-----
Design Method = MWFRS
Input pressure = 10.2 psf
Exposure Category = Open
Building Category = Category 2
End Zone = No
Apply to left end vertical = Yes
Apply to right end vertical = Yes

-----Additional Design Checks-----
2000 lb TC Safe Load: No
Unbalanced Snow Loads: Yes

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1	-	1	0.35
Max CSI in BC PANEL	7	-	8	0.17
Max CSI in Web	6	-	1	0.13

TC	Mem...	Ten	Comp	.CSI.
	OH- 1	179	0	0.35
	1- 2	17	1011	0.26
	2- 3	156	810	0.18
	3- 4	151	810	0.19
	4- 5	15	943	0.18
BC	6- 7	206	255	0.07
	7- 8	626	28	0.17
	8- 9	599	34	0.16
	9-10	255	206	0.07
Web	1- 6	0	1261	0.13
	1- 7	665	0	0.10
	2- 7	96	97	0.02
	2- 8	202	316	0.13
	3- 8	591	123	0.09
	4- 8	199	315	0.13
	4- 9	79	151	0.04
	5- 9	665	0	0.10
	5-10	0	1024	0.11

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
6	0	1326	0	05-08	02-10	SPF
10	13-08-08	1083	0	01-08	HGR	SPF
Max Horiz	=	-207	/	+255	at Joint	6
Max Horiz	=	-207	/	+255	at Joint	10

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
6	Pinned (Wall)	463	137	-110	395
10	H Roll (Hanger)	358	137	-82	326

Loads Summary

See Loadcase Report for load combinations and additional details.
Dead loads have been slope adjusted for any pitch 12.00/12 or greater

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012 (Jan 2020 update).
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements.
Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.01)	7- 8
Vert DL	L/360	L/999(-0.01)	8- 9
Vert TL	L/180	L/999(-0.02)	7- 8
Horz TL	1.00in	(0.00)	@Jt10
Ohng TL	OL/120	OL/441(-0.10)	1- 1

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single: 8- 3

Plate offsets (X, Y):

(None unless indicated below)

Plate Info

Plate	Grip psi		Shear pli			Tens.Pli	
AS	Max	Min	@0	@45	@90	@0	@90
20G	341	249	753	637	589	1144	1130

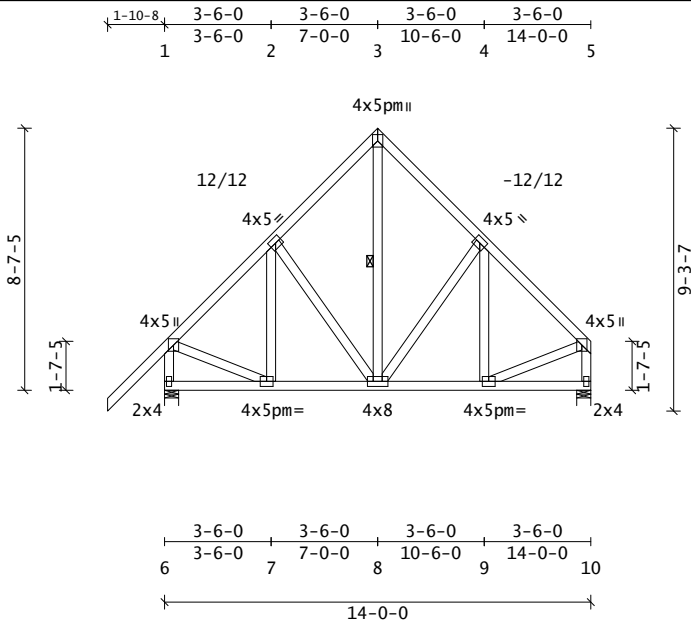
Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with CCMC 13326-L & 13418-L. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

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Truss Mfr. Contact:



Truss Weight = 92.0 lb

Building Code: NBCC2015/TPIC2014
Building Category: Part4 - Gyp ceiling
Importance Category: Normal
Loading(psf) Factor Kh applied: Yes
TCSL(S) 26.2 Dur Fac Kd = 1.00 (Snow)
TCLL 0.0 Kd = 1.00 (Live)
TCDL 10.0 Kd = 1.15 (Wind)
BCLL 10.0
BCDL 10.0
Spacing: 2-00-00 oc Plies: 1

-----Snow Load Specs-----
Ground Snow Ss = 43.9 psf
Rain Load Sr = 4.2 psf
Importance Factor Is = 1.00
Basic roof snow load factor Cb = 0.80
Wind exposure Cw = 1.00
Slope factor Cs = From Geometry
Slippery roof factor = No
Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II

-----Wind Load Specs-----
Design Method = MWFRS
Input pressure = 10.2 psf
Exposure Category = Open
Building Category = Category 2
End Zone = No
Apply to left end vertical = Yes
Apply to right end vertical = Yes

-----Additional Design Checks-----
2000 lb TC Safe Load: No
Unbalanced Snow Loads: Yes

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1	-	1	0.35
Max CSI in BC PANEL	7	-	8	0.17
Max CSI in Web	8	-	4	0.16

TC	Mem...	Ten	Comp	.CSI.
	OH- 1	179	0	0.35
	1- 2	15	1035	0.26
	2- 3	156	837	0.18
	3- 4	148	840	0.20
	4- 5	12	1032	0.20
BC	6- 7	217	253	0.07
	7- 8	643	22	0.17
	8- 9	654	26	0.17
	9-10	253	217	0.07
Web	1- 6	0	1284	0.13
	1- 7	683	0	0.11
	2- 7	94	101	0.02
	2- 8	202	313	0.13
	3- 8	633	126	0.10
	4- 8	201	373	0.16
	4- 9	96	107	0.03
	5- 9	695	0	0.11
	5-10	0	1041	0.10

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
6	0	1349	0	05-08	02-11	SPF
10	14-00-00	1106	0	05-08	02-03	SPF
Max Horiz	=	-217	/	+254	at Joint	6
Max Horiz	=	-217	/	+254	at Joint	10

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
6	Pinned (Wall)	470	140	-113	402
10	H Roll (Wall)	366	139	-86	333

Loads Summary

See Loadcase Report for load combinations and additional details.
Dead loads have been slope adjusted for any pitch 12.00/12 or greater

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements.
Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.01)	8- 9
Vert DL	L/360	L/999(-0.01)	8- 9
Vert TL	L/180	L/999(-0.02)	8- 9
Horz TL	1.00in	(0.00)	@Jt10
Ohng TL	OL/120	OL/442(-0.10)	1- 1

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single: 8- 3

Plate offsets (X, Y):

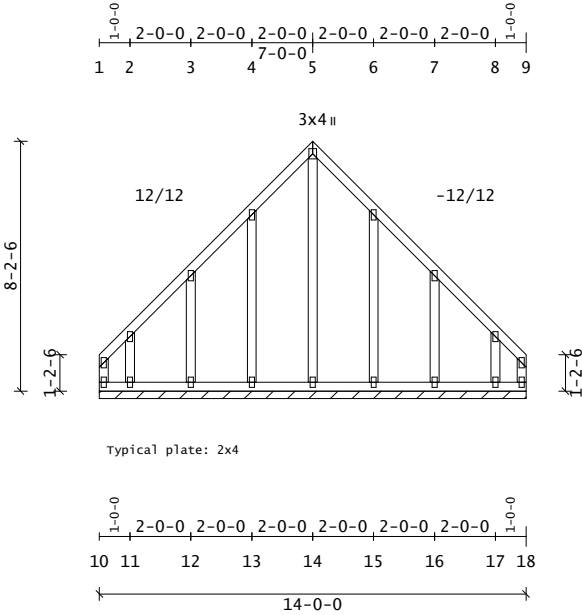
(None unless indicated below)

Plate Info

Plate	Grip psi		Shear pli			Tens.Pli	
AS	Max	Min	@0	@45	@90	@0	@90
20G	341	249	753	637	589	1144	1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with CCMC 13326-L & 13418-L. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.



Truss Weight = 79.3 lb

Building Code: NBCC2015/TPIC2014	-----Snow Load Specs-----	-----Wind Load Specs-----	-----Additional Design Checks-----
Building Category: Part4 - Gyp ceiling	Ground Snow Ss = 43.9 psf	Design Method = MWFRS	2000 lb TC Safe Load: No
Importance Category: Normal	Rain Load Sr = 4.2 psf	Input pressure = 10.2 psf	Unbalanced Snow Loads: Yes
Loading(psf) Factor Kh applied: Yes	Importance Factor Is = 1.00	Exposure Category = Open	
TCSL(S) 26.2 Dur Fac Kd = 1.00 (Snow)	Basic roof snow load factor Cb = 0.80	Building Category = Category 2	
TCLL 0.0 Kd = 1.00 (Live)	Wind exposure Cw = 1.00	End Zone = No	
TCDL 10.0 Kd = 1.15 (Wind)	Slope factor Cs = From Geometry	Apply to left end vertical = Yes	
BCLL 10.0	Slippery roof factor = No	Apply to right end vertical = Yes	
BCDL 10.0	Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II		
Spacing: 2-00-00 oc Plies: 1			

Material Summary				
TC	2x4	SPF	#2	
BC	2x4	SPF	#2	
Webs	2x4	SPF	#2	

Member Forces Summary				
Max CSI in TC PANEL	3	-	4	0.08
Max CSI in BC PANEL	10	-	11	0.10
Max CSI in Web	14	-	5	0.32

...	Mem...	Ten	Comp	.CSI.
TC	1- 5	203	109	0.08
	5- 9	203	109	0.08
BC	10-18	146	70	0.10
Web	1-10	95	207	0.08
	2-11	171	212	0.02
	3-12	149	263	0.05
	4-13	155	289	0.12
	5-14	0	370	0.32
	6-15	155	289	0.12
	7-16	149	263	0.05
	8-17	171	212	0.02
	9-18	95	207	0.08

Maximum Factored Reaction Summary						
-----Reaction Summary(Lbs)-----						
Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
11	1-00-00	284	202	14-00-00		
14	7-00-00	407	0	14-00-00		
17	13-00-00	284	202	14-00-00		
Max Horiz = -213 / +213 at Joint 14						
Reactions not shown: down < 400 and up < 150						
---- Reaction Summary (plf) ----						
Jnt-Jnt	React	-Up-	--Width-			
10- 18	89	0	14-00-00	(reduced)		

Unfactored Reaction Summary					
Jnt	Type	Snow	Live	Wind	Dead
	Pinned (Wall)	732	280	-203	675

Loads Summary
See Loadcase Report for load combinations and additional details.
Dead loads have been slope adjusted for any pitch 12.00/12 or greater

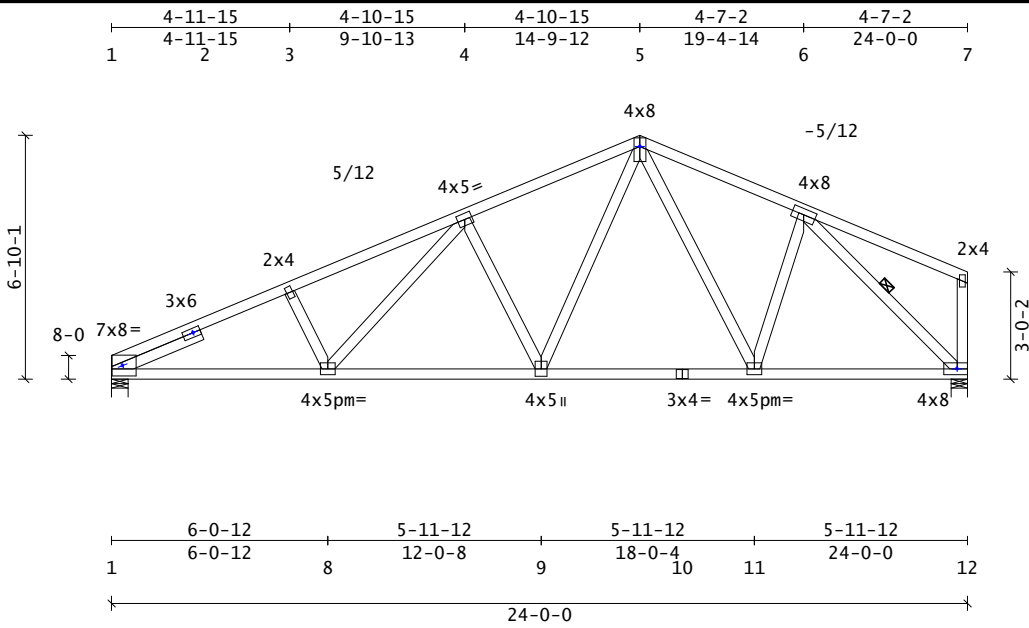
Notes
Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
The maximum rake overhang length is 12.0".
Gable requires 7/16" OSB sheathing on front from 0-00-00 to 14-00-00; connection details to be provided by the Building Designer.
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Lumber and plating have been applied symmetrically.

Deflection Summary				
TrussSpan	Limit	Actual(in)	Location	
Vert LL	L/360	L/999(-0.00)	11-12	
Vert DL	L/360	L/999(-0.00)	11-12	
Vert TL	L/180	L/999(-0.00)	11-12	
Horz TL	1.00in	(0.00)	@Jt10	
Bracing Data Summary				
-----Bracing Data-----				
Chords; continuous except where shown				
Web Bracing -- None				

Plate offsets (X, Y):
(None unless indicated below)

Plate Info									
Plate	Grip	psi	Shear	pli	Tens.	Pli			
AS	Max	Min	@0	@45	@90	@0	@90		
20G	341	249	753	637	589	1144	1130		

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)



Truss Weight = 118.9 lb

Building Code: NBCC2015/TPIC2014	-----Snow Load Specs-----	-----Wind Load Specs-----	-----Additional Design Checks-----
Building Category: Part4 - Gyp ceiling	Ground Snow Ss = 43.9 psf	Design Method = MWFRS	2000 lb TC Safe Load: No
Importance Category: Normal	Rain Load Sr = 4.2 psf	Input pressure = 10.2 psf	Unbalanced Snow Loads: Yes
Loading(psf) Factor Kh applied: Yes	Importance Factor Is = 1.00	Exposure Category = Open	
TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow)	Basic roof snow load factor Cb = 0.80	Building Category = Category 2	
TCLL 0.0 Kd = 1.00 (Live)	Wind exposure Cw = 1.00	End Zone = No	
TCDL 10.0 Kd = 1.15 (Wind)	Slope factor Cs = From Geometry	Apply to left end vertical = Yes	
BCLL 10.0	Slippery roof factor = No	Apply to right end vertical = Yes	
BCDL 10.0	Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II		
Spacing: 2-00-00 oc Plies: 1			

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2
Slider	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	3 - 4	0.68
Max CSI in BC PANEL	1 - 8	0.76
Max CSI in Web	6 - 12	0.54

...	Mem...	Ten	Comp	.CSI.
TC	1- 2	0	2258	0.25
	2- 3	53	4082	0.39
	3- 4	78	3899	0.68
	4- 5	86	2874	0.60
	5- 6	71	2278	0.49
	6- 7	181	107	0.61
BC	1- 8	3681	68	0.76
	8- 9	3022	0	0.66
	9-10	1948	0	0.49
	10-11	1948	0	0.53
	11-12	1858	0	0.51
Web	1- 2	68	2014	0.24
	3- 8	172	494	0.06
	4- 8	902	69	0.14
	4- 9	237	1352	0.40
	5- 9	1598	113	0.25
	5-11	388	384	0.28
	6-11	613	203	0.10
	6-12	0	2665	0.54
	7-12	79	278	0.05

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	0	2317	12	05-08	03-03	SPF
12	24-00-00	2255	0	05-08	03-08	SPF
Max Horiz = -61 / +129 at Joint 1						

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
1	Pinned (Wall)	985	239	-305	479
12	H Roll (Wall)	943	240	-323	480

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements. Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.13)	8- 9
Vert DL	L/360	L/999(-0.08)	8- 9
Vert TL	L/180	L/999(-0.21)	8- 9
Horz TL	1.00in	(0.07) @Jt12	

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single: 6-12

Plate offsets (X, Y):

(None unless indicated below)
Jnt2(-00-07,-00-03), Jnt5(0,-01-01),
Jnt12(-00-08,0), Jnt1(00-08,0)

Plate Info

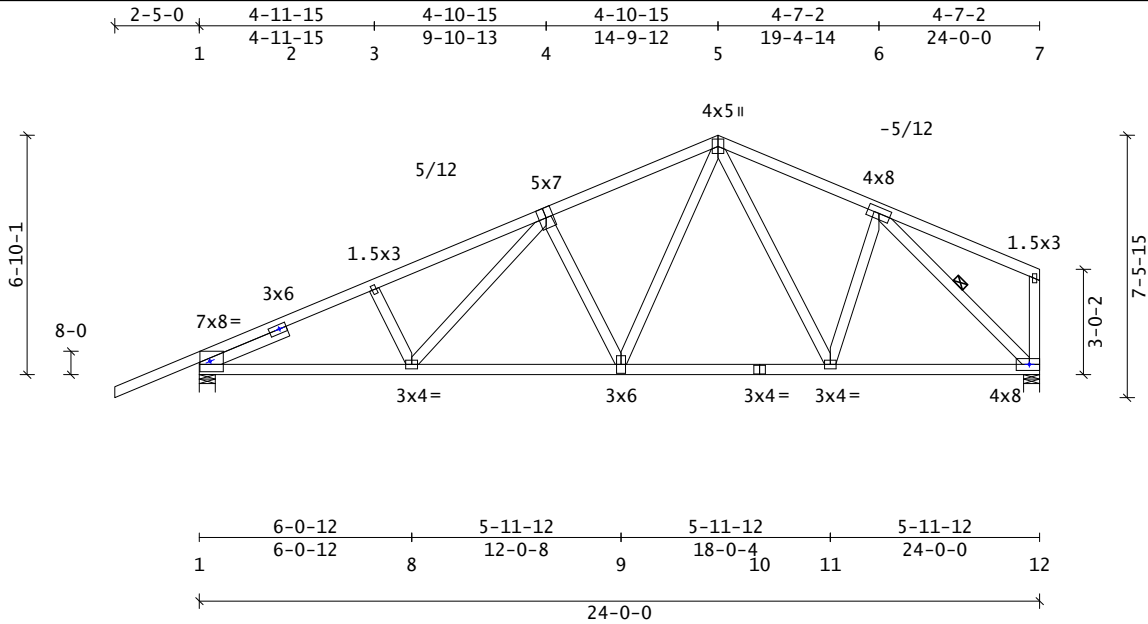
Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Mfr. Contact:



Truss includes non-inventory materials.

Truss Weight = 119.9 lb

Building Code: NBCC2015/TPIC2014 Building Category: Part4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCDL 10.0 Kd = 1.15 (Wind) BCLL 10.0 BCDL 10.0 Spacing: 2-00-00 oc Plies: 1	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes
--	---	---	--

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2
Slider	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	4	-	5	0.79
Max CSI in BC PANEL	1	-	8	0.89
Max CSI in Web	6	-	12	0.54

	Mem...	Ten	Comp	.CSI.
TC	OH-	162	0	0.72
	1-2	0	2105	0.40
	2-3	50	4069	0.55
	3-4	74	3899	0.76
	4-5	66	2792	0.79
	5-6	72	2273	0.49
BC	6-7	181	107	0.61
	1-8	3673	66	0.89
	8-9	2903	0	0.65
	9-10	1943	0	0.50
	10-11	1943	0	0.52
	11-12	1854	0	0.51
Web	1-2	108	2581	0.31
	3-8	198	554	0.07
	4-8	928	95	0.15
	4-9	206	1168	0.34
	5-9	1445	87	0.23
	5-11	390	375	0.27
	6-11	601	204	0.09
	6-12	0	2659	0.54
	7-12	79	278	0.05

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	0	2760	38	05-08	03-13	SPF
12	24-00-00	2251	0	05-08	03-08	SPF
Max Horiz = -38 / +157 at Joint 1						

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
1	Pinned (Wall)	1229	239	-337	540
12	H Roll (Wall)	943	239	-319	477

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Plate marked as unavailable in catalogue: 3 - 1.5x3 Plate marked as unavailable in catalogue: 7 - 1.5x3

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.14)	8- 9
Vert DL	L/360	L/999(-0.09)	8- 9
Vert TL	L/180	L/999(-0.23)	8- 9
Horz TL	1.00in	(0.07)	@Jt12
Ohng TL	OL/120	OL/243(-0.24)	1- 1

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single: 6-12

Plate offsets (X, Y):

(None unless indicated below)
Jnt2(-00-07, -00-03), Jnt12(-00-08,0),
Jnt1(00-08,0)

Plate Info

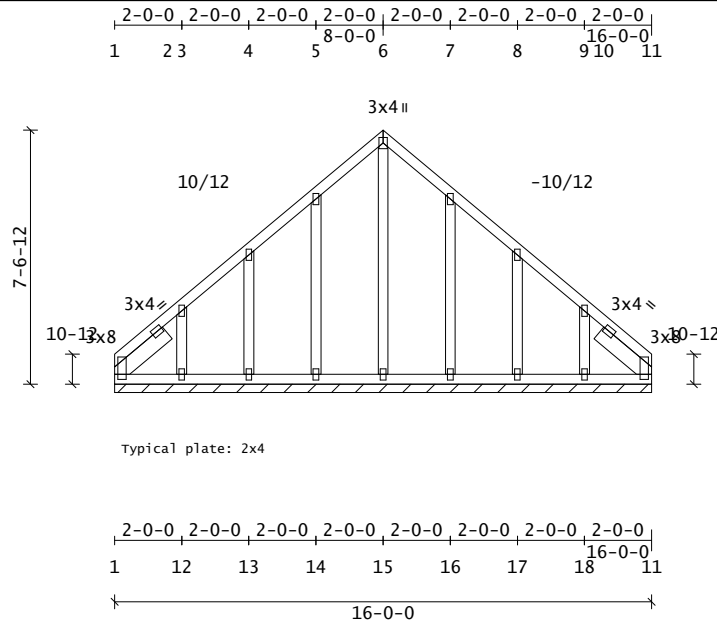
Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with CCMC 13326-L & 13418-L. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.

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Truss Weight = 90.4 lb

Building Code: NBCC2015/TPIC2014	-----Snow Load Specs-----	-----Wind Load Specs-----	-----Additional Design Checks-----
Building Category: Part4 - Gyp ceiling	Ground Snow Ss = 43.9 psf	Design Method = MMFRS	2000 lb TC Safe Load: No
Importance Category: Normal	Rain Load Sr = 4.2 psf	Input pressure = 10.2 psf	Unbalanced Snow Loads: Yes
Loading(psf) Factor Kh applied: Yes	Importance Factor Is = 1.00	Exposure Category = Open	
TCSL(S) 30.7 Dur Fac Kd = 1.00 (Snow)	Basic roof snow load factor Cb = 0.80	Building Category = Category 2	
TCLL 0.0 Kd = 1.00 (Live)	Wind exposure Cw = 1.00	End Zone = No	
TCDL 10.0 Kd = 1.15 (Wind)	Slope factor Cs = From Geometry	Apply to left end vertical = Yes	
BCLL 10.0	Slippery roof factor = No	Apply to right end vertical = Yes	
BCDL 10.0	Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II		
Spacing: 2-00-00 oc Plies: 1			

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.00)	12-13
Vert DL	L/360	L/999(-0.00)	12-13
Vert TL	L/180	L/999(-0.00)	12-13
Horz TL	1.00in	(0.00)	@Jt 1

```

---- Reaction Summary (plf) ----
Jnt-Jnt      React -Up- --Width-
  1- 11      119    0 16-00-00 (reduced)

```

Jnt	Type	Snow	Live	Wind	Dead
	Pinned (Wall)	982	320	-224	640

See Loadcase Report for load combinations and additional details.

Plate	Grip	psi	Shear pli			Tens.Pli	
AS	Max	Min	@0	@45	@90	@0	@90
20G	341	249	753	637	589	1144	1130

Designed as per NCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012 (Jan 2020 update).

If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.

The maximum rake overhang length is 12.0".

Gable requires 7/16" OSB sheathing on front from 0-00-00 to 16-00-00; connection details to be provided by the Building Designer.

Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Lumber and plating have been applied symmetrically.

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

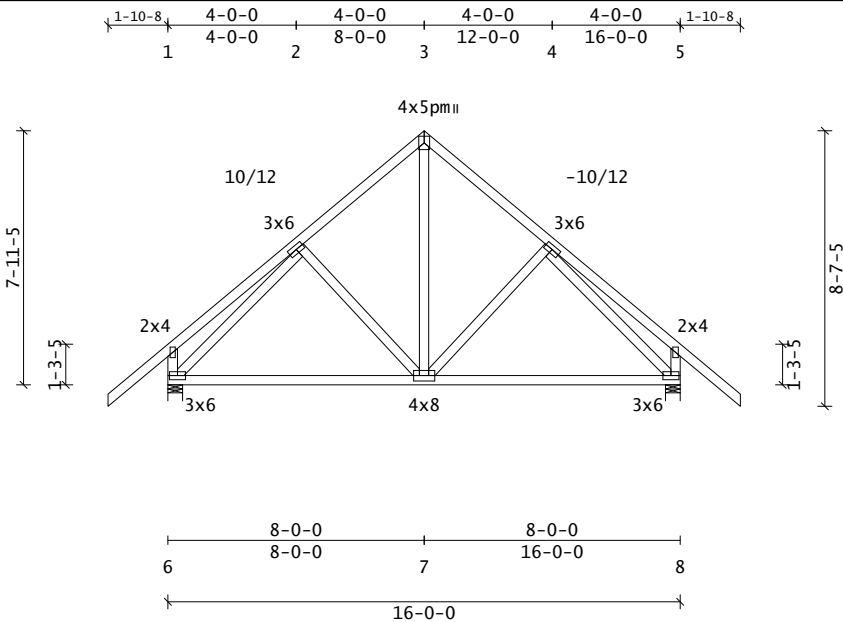
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Truss Studio V
2024.3.2.1

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Truss Mfr. Contact:



Truss Weight = 87.6 lb

Building Code: NBCC2015/TPIC2014		-----Snow Load Specs-----		-----Wind Load Specs-----		-----Additional Design Checks-----	
Building Category: Part4 - Gyp ceiling		Ground Snow Ss = 43.9 psf		Design Method = MWFRS		2000 lb TC Safe Load: No	
Importance Category: Normal		Rain Load Sr = 4.2 psf		Input pressure = 10.2 psf		Unbalanced Snow Loads: Yes	
Loading(psf) Factor Kh applied: Yes		Importance Factor Is = 1.00		Exposure Category = Open			
TCSL(S) 30.7 Dur Fac Kd = 1.00 (Snow)		Basic roof snow load factor Cb = 0.80		Building Category = Category 2			
TCLL 0.0 Kd = 1.00 (Live)		Wind exposure Cw = 1.00		End Zone = No			
TCDL 10.0 Kd = 1.15 (Wind)		Slope factor Cs = From Geometry		Apply to left end vertical = Yes			
BCLL 10.0		Slippery roof factor = No		Apply to right end vertical = Yes			
BCDL 10.0		Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II					
Spacing: 2-00-00 oc Plies: 1							

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1	-	1	0.36
Max CSI in BC PANEL	6	-	7	0.70
Max CSI in Web	6	-	2	0.52

...		Mem...	Ten	Comp	.CSI.
TC	OH-	1	170	0	0.36
	1	2	231	184	0.30
	2	3	110	1120	0.25
	3	4	110	1120	0.25
	4	5	231	184	0.30
BC	5-OH	170	0	0.36	
	6	7	902	47	0.70
	7	8	902	0	0.70
Web	1	6	175	553	0.05
	2	6	0	1319	0.52
	2	7	256	312	0.13
	3	7	838	41	0.13
	4	7	256	312	0.13
	4	8	0	1319	0.52
	5	8	175	553	0.05

Maximum Factored Reaction Summary

-----Reaction Summary(Lbs)-----						
Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
6	0	1526	0	05-08	02-06	SPF
8	16-00-00	1526	0	05-08	02-06	SPF
Max Horiz = -241 / +241 at Joint 6						

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
6	Pinned (Wall)	606	160	-108	364
8	H Roll (Wall)	606	159	-108	364

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements. Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.12)	6- 7
Vert DL	L/360	L/999(-0.12)	6- 7
Vert TL	L/180	L/771(-0.23)	6- 7
Horz TL	1.00in	(0.02)	@Jt 8
Ohng TL	OL/120	OL/567(-0.08)	1- 1
Ohng TL	OL/120	OL/564(-0.08)	5- 5

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)

Plate Info

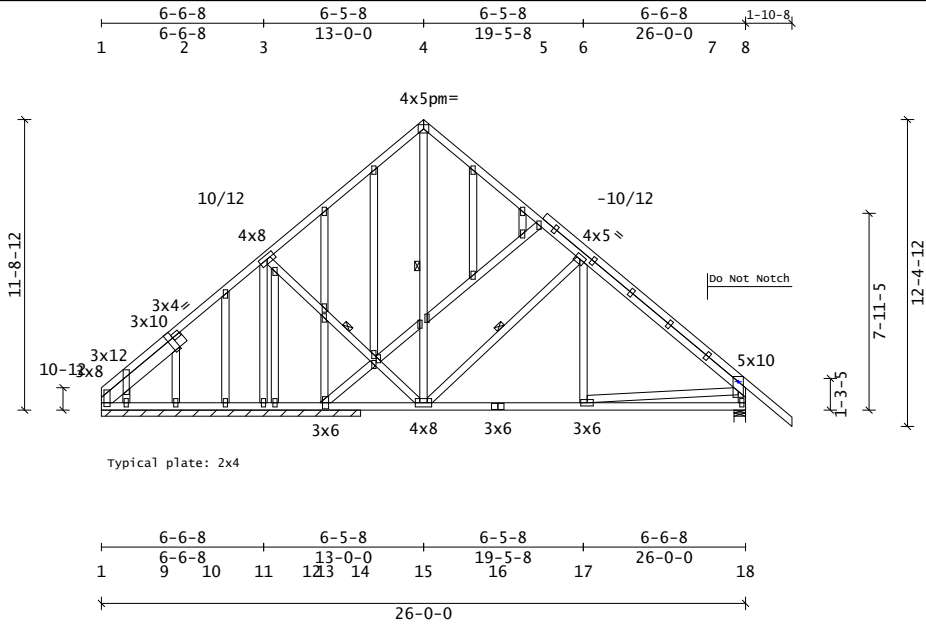
Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

Customer: GREEN-R-PANEL

SID:
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Truss Mfr. Contact:



Truss Weight = 220.2 lb

Building Code: NBCC2015/TPIC2014
Building Category: Part4 - Gyp ceiling
Importance Category: Normal
Loading(psf) Factor Kh applied: Yes
TCSL(S) 30.7 Dur Fac Kd = 1.00 (Snow)
TCLL 0.0 Kd = 1.00 (Live)
TCDL 10.0 Kd = 1.15 (Wind)
BCDL 10.0
BCDL 10.0
Spacing: 2'-00-00 oc Plies: 1

-----Snow Load Specs-----
Ground Snow Ss = 43.9 psf
Rain Load Sr = 4.2 psf
Importance Factor Is = 1.00
Basic roof snow load factor Cb = 0.80
Wind exposure Cw = 1.00
Slope factor Cs = From Geometry
Slippery roof factor = No
Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II

-----Wind Load Specs-----
Design Method = MWFRS
Input pressure = 10.2 psf
Exposure Category = Open
Building Category = Category 2
End Zone = No
Apply to left end vertical = Yes
Apply to right end vertical = Yes

-----Additional Design Checks-----
2000 lb TC Safe Load: No
Unbalanced Snow Loads: Yes

Material Summary

UTC	2x4	SPF	#2
LTC	2x4	SPF	#2
TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2
Slider	2x6	SPF	#2
FC	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	6	-	8	0.80
Max CSI in BC PANEL	16	-	17	0.52
Max CSI in Web	11	-	3	0.74

	Mem...	Ten	Comp	.CSI.
TC	1- 3	264	581	0.72
	3- 4	299	1079	0.73
	4- 6	294	1085	0.78
	6- 8	113	1850	0.80
	8-OH	170	0	0.36
BC	1- 9	427	345	0.11
	9-10	427	345	0.08
	10-11	427	345	0.06
	11-12	427	345	0.06
	12-14	427	345	0.12
	14-15	427	345	0.17
	15-16	1264	0	0.34
	16-17	1264	0	0.52
	17-18	0	0	0.27
Web	3-11	13	1589	0.74
	3-15	679	17	0.11
	4-15	428	239	0.17
	6-15	316	1026	0.39
	6-17	358	0	0.05
	8-17	1268	0	0.20
	8-18	111	1768	0.17

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
18	26-00-00	1884	65	05-08	03-12	SPF
1	04-00	660	155	10-05-08		
11	6-06-08	1689	0	10-05-08		
Max Horiz = -371 / +339 at Joint 1						
Reactions not shown: down < 400 and up < 150						
Jnt	Jnt	React	-Up-	--Width-		
1- 14	27	0	10-05-08	(reduced)		

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
	Pinned (Wall)	981	338	-220	657
18	H Roll (Wall)	778	181	-143	427

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.
The maximum rake overhang length is 12.0".
Gable requires 7/16" OSB sheathing on front from 0-00-00 to 26-00-00; connection details to be provided by the Building Designer.
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements.
Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBACE.
The upper top chord (UTC) may be notched 1.5" deep x 3.5" wide at 24"o.c. max. for outlookers. Do not notch in the heel areas marked or anywhere there is a single chord member. Do not cut the connector plates.
Attach stacked chords with 2x4 20 ga. plates, u.n.o.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.07)	17-18
Vert DL	L/360	L/999(-0.07)	17-18
Vert TL	L/180	L/999(-0.13)	17-18
Horz TL	1.00in	(0.01)	@Jt18
Ohng TL	OL/120	OL/912(-0.05)	8- 8

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single: 3-15 15- 4 15- 6

Plate offsets (X, Y):

(None unless indicated below)
Jnt8(0,-02-08)

Plate Info

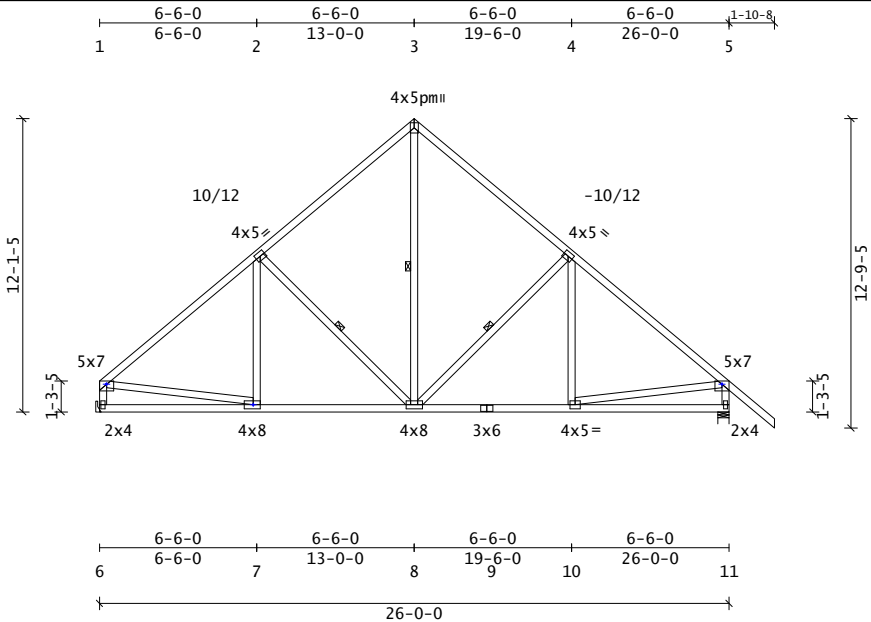
Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

Customer: GREEN-R-PANEL

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Truss Mfr. Contact:



Truss Weight = 143.7 lb

Building Code: NBCC2015/TPIC2014
Building Category: Part4 - Gyp ceiling
Importance Category: Normal
Loading(psf) Factor Kh applied: Yes
TCSL(S) 30.7 Dur Fac Kd = 1.00 (Snow)
TCLL 0.0 Kd = 1.00 (Live)
TCDL 10.0 Kd = 1.15 (Wind)
BCLL 10.0
BCDL 10.0
Spacing: 2-00-00 oc Plies: 1

-----Snow Load Specs-----
Ground Snow Ss = 43.9 psf
Rain Load Sr = 4.2 psf
Importance Factor Is = 1.00
Basic roof snow load factor Cb = 0.80
Wind exposure Cw = 1.00
Slope factor Cs = From Geometry
Slippery roof factor = No
Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II

-----Wind Load Specs-----
Design Method = MWFRS
Input pressure = 10.2 psf
Exposure Category = Open
Building Category = Category 2
End Zone = No
Apply to left end vertical = Yes
Apply to right end vertical = Yes

-----Additional Design Checks-----
2000 lb TC Safe Load: No
Unbalanced Snow Loads: Yes

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1	-	2	0.87
Max CSI in BC PANEL	7	-	8	0.56
Max CSI in Web	2	-	8	0.38

...	Mem...	Ten	Comp	.CSI.
TC	1- 2	10	2447	0.87
	2- 3	199	1830	0.85
	3- 4	199	1829	0.80
	4- 5	13	2445	0.78
	5-OH	170	0	0.36
BC	6- 7	366	336	0.26
	7- 8	1738	0	0.56
	8- 9	1732	0	0.47
	9-10	1732	0	0.55
	10-11	0	0	0.26
Web	1- 6	15	1988	0.20
	1- 7	1756	0	0.28
	2- 7	245	62	0.04
	2- 8	321	933	0.38
	3- 8	1377	80	0.22
	4- 8	319	893	0.36
	4-10	245	60	0.04
	5-10	1750	0	0.28
	5-11	38	2227	0.22

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
6	0	2105	0	01-08	HGR	SPF
11	26-00-00	2344	0	05-08	04-11	SPF
Max Horiz = -366 / +336 at Joint 6						

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
6	Pinned(Hanger)	798	260	-177	518
11	H Roll (Wall)	917	260	-174	566

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
A "pm" next to the plate size indicates that the plate has been user modified; see Plate Offsets for any special positioning requirements.
Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.08)	6- 7
Vert DL	L/360	L/999(-0.07)	6- 7
Vert TL	L/180	L/999(-0.15)	6- 7
Horz TL	1.00in	(0.02) @Jt11	
Ohng TL	OL/120	OL/211(0.21)	5- 5

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single: 2- 8 8- 3 8- 4

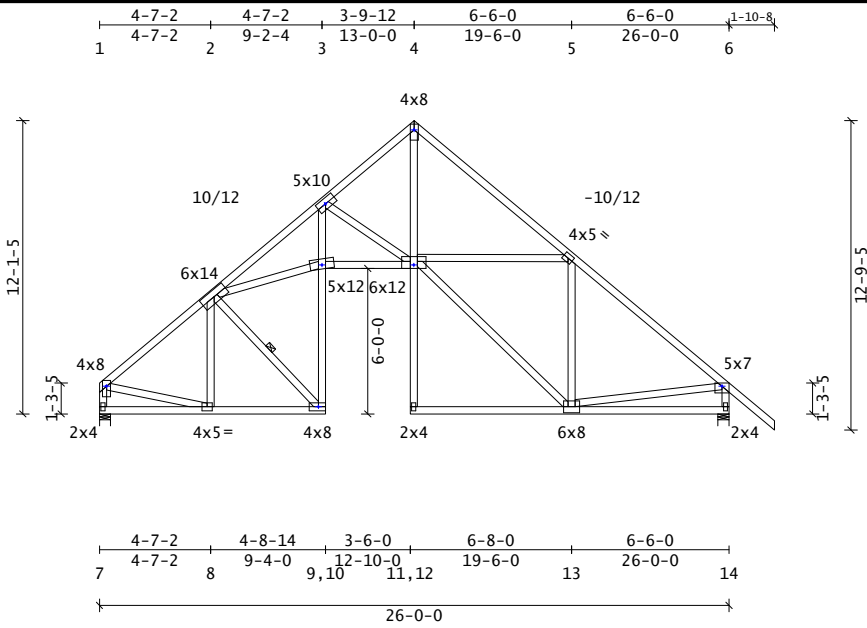
Plate offsets (X, Y):

(None unless indicated below)
Jnt1(0,-00-14), Jnt5(0,-00-14),
Jnt7(-00-08,0)

Plate Info

Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)



Truss Weight = 182.5 lb

Building Code: NBCC2015/TPIC2014 Building Category: Part4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 30.7 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCDL 10.0 Kd = 1.15 (Wind) BCLL 10.0 BCDL 10.0 Spacing: 2-00-00 oc Plies: 1	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes
--	---	---	--

Material Summary

TC	2x4	SPF	1650/1.5
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	4 - 5	0.74
Max CSI in BC PANEL	10 - 11	0.89
Max CSI in Web	2 - 10	0.80

...	Mem...	Ten	Comp	.CSI.
TC	1- 2	0	2402	0.30
	2- 3	0	6465	0.55
	3- 4	0	3252	0.23
	4- 5	0	3370	0.74
	5- 6	9	2452	0.62
	6-OH	170	0	0.30
BC	7- 8	366	336	0.14
	8- 9	1739	23	0.40
	10-11	4856	0	0.89
	12-13	4	0	0.38
	13-14	0	0	0.29
Web	1- 7	0	2021	0.20
	1- 8	1775	0	0.28
	2- 8	148	175	0.04
	2- 9	34	2484	0.53
	2-10	4973	0	0.80
	3-10	3246	0	0.65
	3-11	114	3008	0.73
	4-11	3457	0	0.59
	5-11	716	65	0.11
	5-13	94	1484	0.80
	6-13	1755	0	0.28
	6-14	35	2233	0.22
	9-10	1872	0	0.43
	11-12	136	0	0.05
	11-13	2354	0	0.38

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
7	0	2105	0	05-08	04-03	SPF
14	26-00-00	2344	0	05-08	04-11	SPF
Max Horiz = -366 / +336 at Joint 7						

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
7	Pinned (Wall)	798	259	-177	518
14	H Roll (Wall)	917	260	-174	566

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE. Vertical Step - Ensure adequate drift loads are applied

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.26)	9-10
Vert DL	L/360	L/999(-0.17)	12-13
Vert TL	L/180	L/706(-0.43)	9-10
Horz TL	1.00in	(0.49) @Jt14	
Ohng TL	OL/120	OL/186(0.24)	6- 6

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single: 2- 9

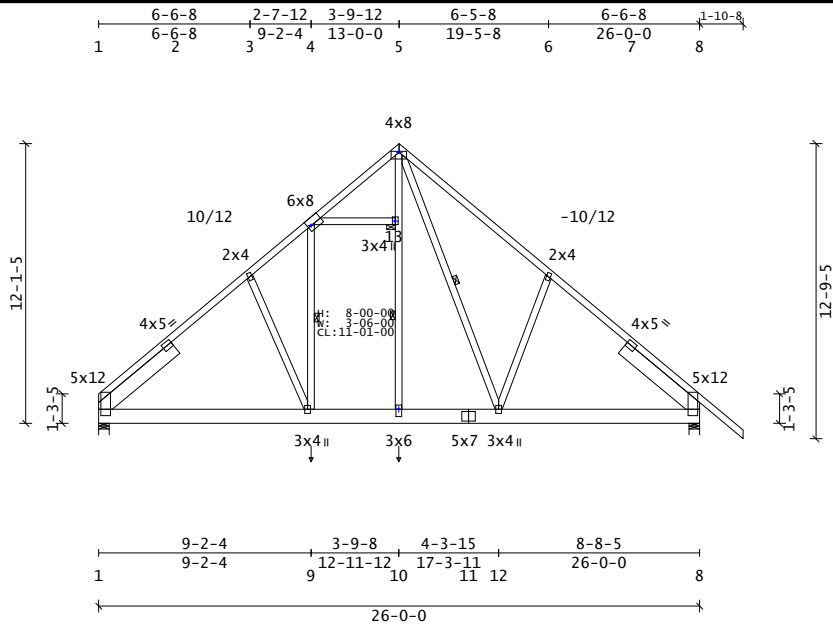
Plate offsets (X, Y):

(None unless indicated below)
Jnt1(0,-01-02), Jnt3(00-05,00-05),
Jnt4(0,-01-02), Jnt6(0,-00-14),
Jnt9(-00-08,0), Jnt10(0,00-08),
Jnt11(0,01-04)

Plate Info

Plate	Grip psi	Shear pli	Tens.Pli
AS	Max Min	@0 @45 @90	@0 @90
20G	341 249	753 637 589	1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)



Truss Weight = 190.5 lb

Building Code: NBCC2015/TPIC2014 Building Category: Part4 - Gyp ceiling Importance Category: Normal Loading(psf) Factor Kh applied: Yes TCSL(S) 30.7 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) TCDL 10.0 Kd = 1.15 (Wind) BCLL 10.0 BCDL 10.0 Spacing: 2-00-00 oc Plies: 1	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes Attic Floor: LL = 40.0 psf, DL = 10.0 psf Attic Wall: DL = 5.0 psf Attic Ceiling: DL = 5.0 psf
--	---	---	---

Material Summary

TC	2x4	SPF	#2	
	2x4	SPF	1650/1.5	5-8
BC	2x8	SPF	1950/1.7	
	2x8	SPF	#2	11-8
Webs	2x4	SPF	#2	
Slider	2x8	SPF	#2	
TB	2x4	SPF	#2	

Member Forces Summary

Max CSI in TC PANEL	5 - 6	0.80
Max CSI in BC PANEL	9 - 10	0.56
Max CSI in Web	10 - 13	0.62

Maximum Factored Reaction Summary

-----Reaction Summary (Lbs)-----
Jnt --X-Loc- React -Up- --Width- -Reqd -Mat
1 0 2476 0 05-08 03-08 SPF
8 26-00-00 2620 0 05-08 03-11 SPF
Max Horiz = -384 / +338 at Joint 1

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
1	Pinned (Wall)	798	434	-174	675
8	H Roll (Wall)	917	389	-171	683

Loads Summary

Attic space centred at 11-01-00 is loaded with 40.0 psf Live & 10.0 psf Dead Floor, 5.0 psf Dead Wall, 5.0 psf Dead Ceiling loads, and meets deflection criteria L/360.

See Loadcase Report for load combinations and additional details.

Unfactored Concentrated Loads (Max/Min)

Mbr	Dead	Snow	Live	Wind	Location	Dir	Desc
Web	80/80	0/0	0/0	0/0	9-02-04	Vert	
SidewallDL							
Web	80/80	0/0	0/0	0/0	12-11-12	Vert	
SidewallDL							

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, CBC2018, MBC2018, and OBC2012(Jan 2020 update).

Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE.

Partial reinforcing member does not meet TPIC Clause B.1(5). Full reinforcing member required

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.24)	1- 9
Vert DL	L/360	L/999(-0.17)	1- 9
Vert TL	L/180	L/732(-0.41)	1- 9
Horz TL	1.00in	(0.08)	@Jt 1
Ohng TL	OL/120	OL/436(-0.10)	8- 8

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Attic tie beam (TB) & walls; bracing indicated or rigid sheathing.
-----Purlins-----
--oc-- --From-- --To-- #Bays
WB3 4-01-00 07-04 8-08-01 2
WB4 5-07-00 07-04 11-07-01 2
----- Web Bracing -- CLR -----
Single: 9- 4 10-13 5-12
Single: Joint 13

Plate offsets (X, Y):

(None unless indicated below)

Jnt4(01-07,01-13), Jnt5(-00-04,-01-07), Jnt10(0,-00-12), Jnt13(0,00-04)

Plate Info

Plate	Grip	psi	Shear	pli	Tens.	Pli
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130

Plate Placement Tol. = 0.250 inches

Plate Rotation Tol. = 5.0 Deg.

Customer: GREEN-R-PANEL

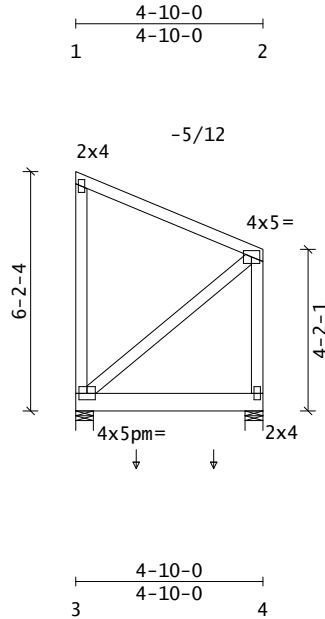
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TID: RB24003
Date: 04 / 02 / 25
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Truss Mfr. Contact:

Plate Grip psi Shear pli Tens.Pli
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

SID:
TID: RB24003
Date: 04 / 02 / 25
Page: 1 of 1

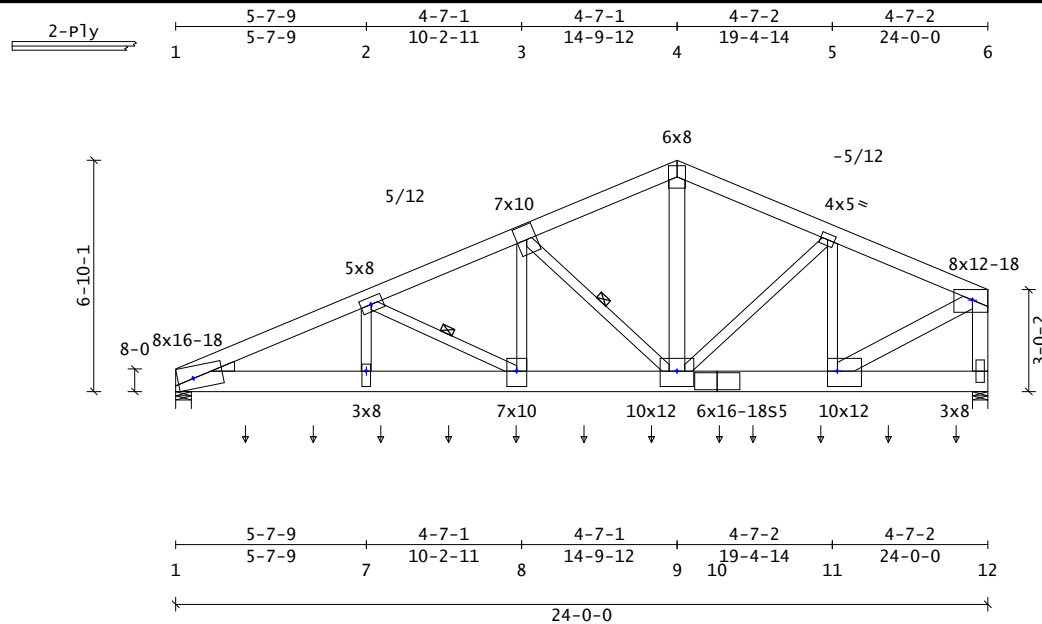
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-----Additional Design Checks-----
2000 lb TC Safe Load:                No
Unbalanced Snow Loads:                 Yes
```

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Weight = 439.6 lb

Building Code: NBCC2015/TPIC2014 Building Category: Part 4 - Gyp ceiling Importance Category: Normal Loading (psf) TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow) TCLL 0.0 Kd = 1.00 (Live) BCLL 10.0 Kd = 1.15 (Wind) BCDL 10.0 Spacing: 1-00-00 oc Plies: 2	-----Snow Load Specs----- Ground Snow Ss = 43.9 psf Rain Load Sr = 4.2 psf Importance Factor Is = 1.00 Basic roof snow load factor Cb = 0.80 Wind exposure Cw = 1.00 Slope factor Cs = From Geometry Slippery roof factor = No Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II	-----Wind Load Specs----- Design Method = MWFRS Input pressure = 10.2 psf Exposure Category = Open Building Category = Category 2 End Zone = No Apply to left end vertical = Yes Apply to right end vertical = Yes	-----Additional Design Checks----- 2000 lb TC Safe Load: No Unbalanced Snow Loads: Yes
--	---	---	--

Material Summary				
TC	2x6	SPF	1650/1.5	
	2x6	SPF	#2	4-6
BC	2x8	SPF	1950/1.7	
Webs	2x4	SPF	#2	
	2x6	SPF	#2	9-4 11-6 12-6
Wedge	2x4	SPF	#2	

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.64
Max CSI in BC PANEL	1 - 7	0.96
Max CSI in Web	11 - 6	0.94

	Mem.	Ten	Comp	C.S.I.
TC	1-2	530	26524	0.64
	2-3	315	20876	0.41
	3-4	72	14614	0.36
	4-5	96	14605	0.53
	5-6	0	13924	0.51
BC	1-7	24429	502	0.96
	7-8	24429	502	0.83
	8-9	19249	255	0.74
	9-10	12786	0	0.59
	10-11	12786	0	0.53
	11-12	17	5	0.28
Web	2-7	5489	140	0.48
	2-8	279	6913	0.55
	3-8	8907	273	0.78
	3-9	379	9485	0.93
	4-9	10839	54	0.70
	5-9	2833	1648	0.37
	5-11	1284	2791	0.31
	6-11	14505	0	0.94
	6-12	0	12363	0.50

Maximum Factored Reaction Summary

```

-----Reaction Summary(Lbs)-----
Jnt  --X-Loc-  React  Up-  --Width-  Req'd  -Mat
   1         0 13236  283    05-08  07-03**  SPF
  12 24-00-00 14519   50    05-08  11-13**  SPF
Max Horiz =   -23 /   +60 at Joint 1
(**) indicates Req'd Width > actual Width; enhancement may be required.
Building Designer to provide adequate bearing size or enhancement.

```

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
1	Pinned (Wall)	5136	3005	-1146	3158
12	H Roll (Wall)	5549	3378	-1281	3537

Loads Summary

See Loadcase Report for load combinations and additional details.

Unfactored Concentrated Loads (Max/Min)		Dead		Snow		Live		Wind Location		Dir	Desc
Mbr		Dead		Snow		Live		Wind Location		Dir	Desc
Transfer loads:											
BC	518/518	798/238	260/260	-177/-386	2-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	4-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	6-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	8-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	10-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	12-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	14-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	16-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	17-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	19-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	21-00-12	Vert	T1				
BC	518/518	798/238	260/260	-177/-386	23-00-12	Vert	T1				

2-PLY TRUSS Fastener Spacing

Tension each ply to the adjacent ply as follows (rows staggered):

FC	2x6,	2-row(s)	of 3	1/4"	Gun Nails @	12.0"	o.c.
BC	2x8,	2-row(s)	of 3	1/4"	Gun Nails @	12.0"	o.c.**
WB	2x4,	1-row(s)	of 3	1/4"	Gun Nails @	9.0"	o.c.
WB	2x6,	2-row(s)	of 3	1/4"	Gun Nails @	9.0"	o.c.

** Use additional fasteners of the same type (u.n.o.) within +/-12" of the location(s) indicated (except where approved hangers are used with fasteners that transfer the load to all plies):

EC:2-00-12, 6, BC:4-00-12, 6, BC:6-00-12, 6
EC:8-00-12, 6, BC:10-00-12, 6, BC:12-00-12, 6
EC:14-00-12, 6, BC:16-06-12, 15, BC:19-00-12, 6
EC:21-00-12, 6, BC:23-00-12, 6
Refer to TD-FRM-0005 for cluster fastening spacing limits.

Notes

Deflection Summary

TrussSpan	Limit	Actual (in)	Location
Vert LL	L/360	L/999 (-0.23)	7- 8
Vert DL	L/360	L/999 (-0.14)	7- 8
Vert TL	L/180	L/762 (-0.36)	7- 8
Horz TL	1.00in	(0.08)	@Jt12

Bracing Data Summary

```
-----Bracing Data-----
Chords; continuous except where shown
----- Web Bracing -- CLR -----
Single:      2- 8  3- 9
```

Plate offsets (X, Y):

(None unless indicated below)
Jnt2(00-03,00-01), Jnt6(-00-08,-00-05),
Jnt7(0,-01-08), Jnt8(0,-00-08),
Jnt9(0,-00-08), Jnt11(02-08,-00-08),
Jnt1(02-06,00-14)

Plate Info

Plate	Grip psi		Shear pli		Tens.Pli	
AS	Max	Min	@0	@45	@90	@0 @90
20G	341	249	753	637	589	1144 1130
HS18G	344	220	1103	890	979	1829 1808
S518G	203	123	1103	890	979	2466 1809

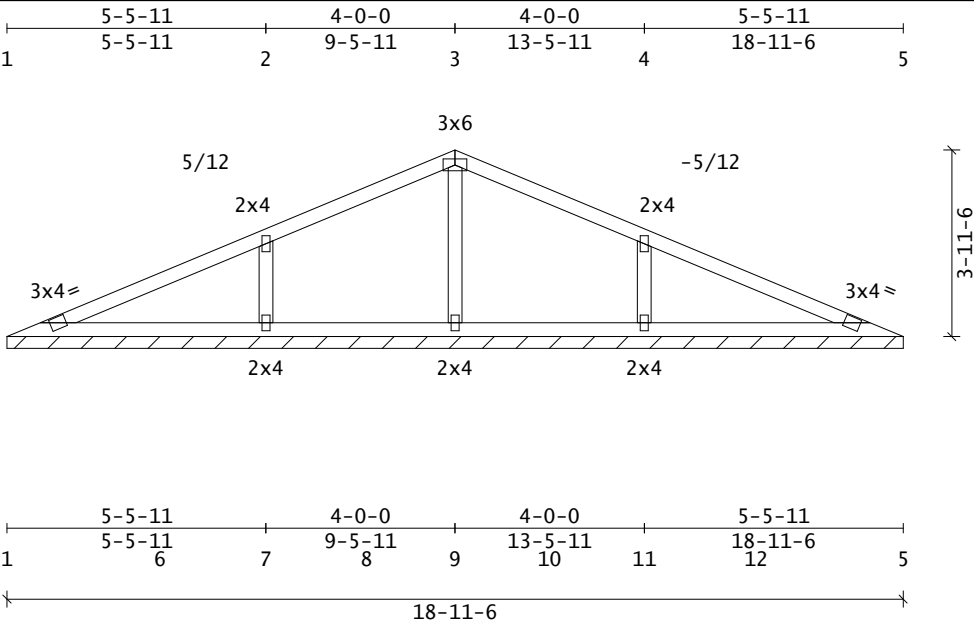
Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Mfr. Contact:

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update).
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.
Continuous Lateral Restraint (CLR) rows require diagonal bracing per D-WEBCLRBRACE.
Bearing @ 02-12 exceeds wall width. Bearing enhancement may be required.
Bearing @ 23-09-04 exceeds wall width. Bearing enhancement may be required.



Truss Weight = 52.4 lb

Building Code: NBCC2015/TPIC2014	-----Snow Load Specs-----	-----Wind Load Specs-----	-----Additional Design Checks-----
Building Category: Part4 - Gyp ceiling	Ground Snow Ss = 43.9 psf	Design Method = MWFRS	2000 lb TC Safe Load: No
Importance Category: Normal	Rain Load Sr = 4.2 psf	Input pressure = 10.2 psf	Unbalanced Snow Loads: Yes
Loading(psf) Factor Kh applied: Yes	Importance Factor Is = 1.00	Exposure Category = Open	
TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow)	Basic roof snow load factor Cb = 0.80	Building Category = Category 2	
TCLL 0.0 Kd = 1.00 (Live)	Wind exposure Cw = 1.00	End Zone = No	
TCDL 10.0 Kd = 1.15 (Wind)	Slope factor Cs = From Geometry	Apply to left end vertical = Yes	
BCLL 10.0	Slippery roof factor = No	Apply to right end vertical = Yes	
BCDL 10.0	Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II		
Spacing: 2-00-00 oc Plies: 1			

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.46
Max CSI in BC PANEL	1 - 6	0.05
Max CSI in Web	7 - 2	0.09

...	Mem...	Ten	Comp	.CSI.
TC	1- 2	112	178	0.46
	2- 3	150	206	0.34
	3- 4	150	206	0.34
	4- 5	112	178	0.46
BC	1- 6	80	8	0.05
	5-12	80	8	0.05
	6- 7	80	8	0.04
	7- 8	80	8	0.03
	8- 9	80	8	0.03
	9-10	80	8	0.03
	10-11	80	8	0.03
	11-12	80	8	0.04
Web	2- 7	196	828	0.09
	3- 9	0	380	0.06
	4-11	196	828	0.09

Maximum Factored Reaction Summary

Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	1-02-10	601	36	18-11-06		
7	5-07-03	923	153	18-11-06		
9	9-07-03	471	0	18-11-06		
11	13-07-03	923	153	18-11-06		
5	17-11-12	601	36	18-11-06		
Max Horiz = -61 / +61 at Joint 9						
Reactions not shown: down < 400 and up < 150						
---- Reaction Summary (plf) ----						
Jnt-Jnt	React	-Up-	--Width-			
1- 5	20	0	18-11-06	(reduced)		

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
	Pinned (Wall)	1490	378	-507	757

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Valley Truss application only. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.00)	1- 6
Vert DL	L/360	L/999(-0.00)	11-12
Vert TL	L/180	L/999(-0.00)	1- 6
Horz TL	1.00in	(0.00) @Jt 1	

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)

Plate Info

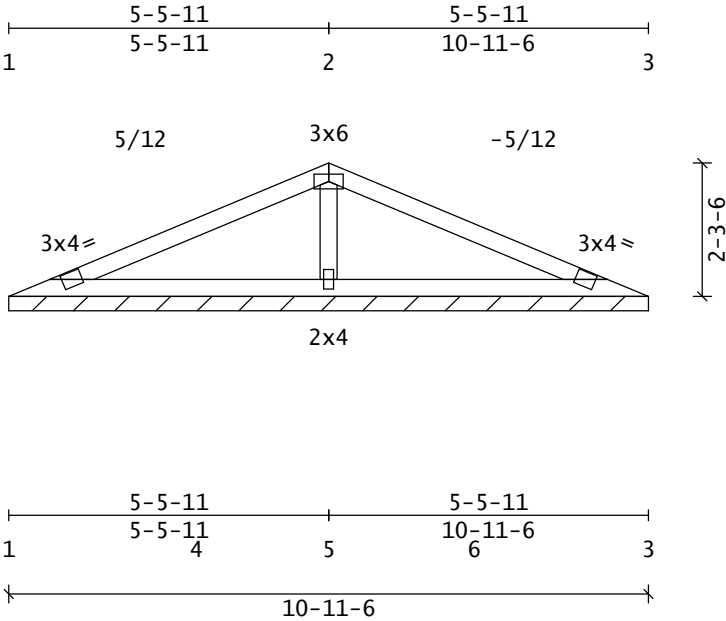
Plate	Grip	psi	Shear	pli	Tens.Pli
AS	Max	Min	@0	@45	@90
20G	341	249	753	637	589 1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Mfr. Contact:



Truss Weight = 26.8 lb

Building Code: NBCC2015/TPIC2014		-----Snow Load Specs-----		-----Wind Load Specs-----		-----Additional Design Checks-----	
Building Category: Part4 - Gyp ceiling		Ground Snow Ss = 43.9 psf		Design Method = MWFRS		2000 lb TC Safe Load:	
Importance Category: Normal		Rain Load Sr = 4.2 psf		Input pressure = 10.2 psf		Unbalanced Snow Loads:	
Loading(psf) Factor Kh applied: Yes		Importance Factor Is = 1.00		Exposure Category = Open			
TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow)		Basic roof snow load factor Cb = 0.80		Building Category = Category 2			
TCLL 0.0 Kd = 1.00 (Live)		Wind exposure Cw = 1.00		End Zone = No			
TCDL 10.0 Kd = 1.15 (Wind)		Slope factor Cs = From Geometry		Apply to left end vertical = Yes			
BCLL 10.0		Slippery roof factor = No		Apply to right end vertical = Yes			
BCDL 10.0		Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II					
Spacing: 2-00-00 oc Plies: 1							

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1	-	2	0.37
Max CSI in BC PANEL	1	-	4	0.16
Max CSI in Web	5	-	2	0.04

...	Mem...	Ten	Comp	.CSI.
TC	1- 2	123	353	0.37
	2- 3	123	353	0.37
BC	1- 4	197	52	0.16
	3- 6	197	52	0.16
	4- 5	197	52	0.08
	5- 6	197	52	0.08
Web	2- 5	8	362	0.04

Maximum Factored Reaction Summary

-----Reaction Summary(Lbs)-----						
Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	1-02-10	691	69	10-11-06		
5	5-07-03	453	0	10-11-06		
3	9-11-12	691	69	10-11-06		
Max Horiz = -35 / +35 at Joint 5						
Reactions not shown: down < 400 and up < 150						
---- Reaction Summary (plf) ----						
Jnt-Jnt	React	-Up-	--Width-			
1- 3	20	0	10-11-06	(reduced)		

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
	Pinned (Wall)	860	218	-292	437

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Valley Truss application only. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.00)	6- 3
Vert DL	L/360	L/999(-0.00)	6- 3
Vert TL	L/180	L/999(-0.00)	6- 3
Horz TL	1.00in	(0.01) @Jt 3	

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)

Plate Info

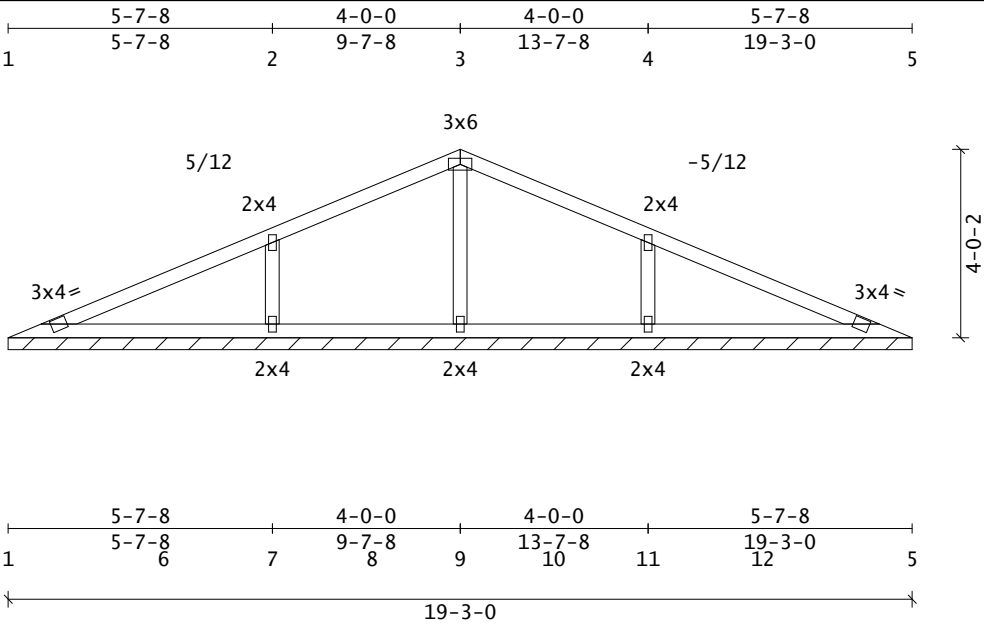
Plate	Grip	psi	Shear	pli	Tens.Pli
AS	Max	Min	@0	@45	@90
20G	341	249	753	637	589 1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Mfr. Contact:



Truss Weight = 53.2 lb

Building Code: NBCC2015/TPIC2014	-----Snow Load Specs-----	-----Wind Load Specs-----	-----Additional Design Checks-----
Building Category: Part4 - Gyp ceiling	Ground Snow Ss = 43.9 psf	Design Method = MWFRS	2000 lb TC Safe Load: No
Importance Category: Normal	Rain Load Sr = 4.2 psf	Input pressure = 10.2 psf	Unbalanced Snow Loads: Yes
Loading(psf) Factor Kh applied: Yes	Importance Factor Is = 1.00	Exposure Category = Open	
TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow)	Basic roof snow load factor Cb = 0.80	Building Category = Category 2	
TCLL 0.0 Kd = 1.00 (Live)	Wind exposure Cw = 1.00	End Zone = No	
TCDL 10.0 Kd = 1.15 (Wind)	Slope factor Cs = From Geometry	Apply to left end vertical = Yes	
BCLL 10.0	Slippery roof factor = No	Apply to right end vertical = Yes	
BCDL 8.0	Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II		
Spacing: 2-00-00 oc Plies: 1			

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.48
Max CSI in BC PANEL	1 - 6	0.07
Max CSI in Web	7 - 2	0.09

...	Mem...	Ten	Comp	.CSI.
TC	1- 2	108	195	0.48
	2- 3	152	219	0.35
	3- 4	152	219	0.35
	4- 5	108	195	0.48
BC	1- 6	90	8	0.07
	5-12	90	8	0.07
	6- 7	90	8	0.05
	7- 8	90	8	0.03
	8- 9	90	8	0.03
	9-10	90	8	0.03
	10-11	90	8	0.03
	11-12	90	8	0.05
Web	2- 7	199	842	0.09
	3- 9	0	365	0.06
	4-11	199	842	0.09

Maximum Factored Reaction Summary

Jnt	--X-Loc	React	-Up-	--Width-	-Reqd	-Mat
1	1-02-10	603	45	19-03-00		
7	5-09-00	927	164	19-03-00		
9	9-09-00	446	0	19-03-00		
11	13-09-00	927	164	19-03-00		
5	18-03-06	603	45	19-03-00		
Max Horiz = -62 / +62 at Joint 9						
Reactions not shown: down < 400 and up < 150						
---- Reaction Summary (plf) ----						
Jnt-Jnt	React	-Up-	--Width-			
1- 5	18	0	19-03-00	(reduced)		

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
	Pinned (Wall)	1513	384	-516	693

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Valley Truss application only. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.00)	1- 6
Vert DL	L/360	L/999(-0.00)	11-12
Vert TL	L/180	L/999(-0.00)	1- 6
Horz TL	1.00in	(0.00) @Jt 1	

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)

Plate Info

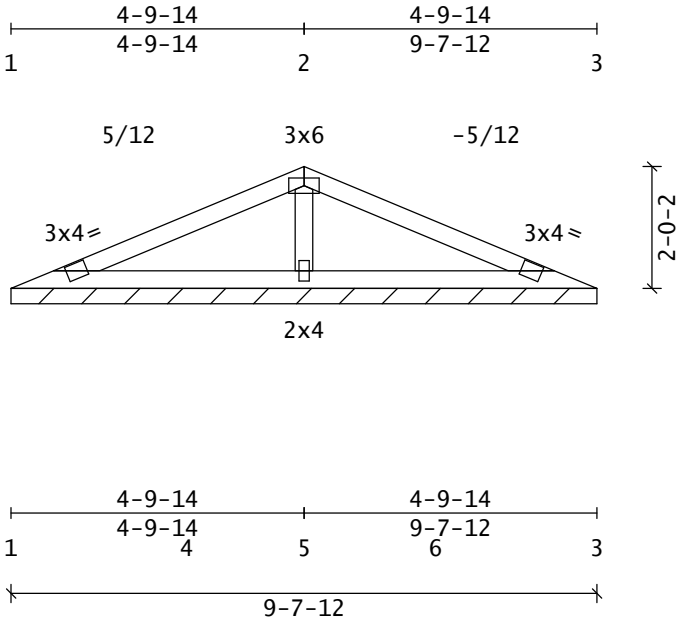
Plate	Grip	psi	Shear	pli	Tens.Pli
AS	Max	Min	@0	@45	@90
20G	341	249	753	637	589 1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)

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Truss Mfr. Contact:



Truss Weight = 23.6 lb

Building Code: NBCC2015/TPIC2014	-----Snow Load Specs-----	-----Wind Load Specs-----	-----Additional Design Checks-----
Building Category: Part4 - Gyp ceiling	Ground Snow Ss = 43.9 psf	Design Method = MWFRS	2000 lb TC Safe Load: No
Importance Category: Normal	Rain Load Sr = 4.2 psf	Input pressure = 10.2 psf	Unbalanced Snow Loads: Yes
Loading(psf) Factor Kh applied: Yes	Importance Factor Is = 1.00	Exposure Category = Open	
TCSL(S) 39.3 Dur Fac Kd = 1.00 (Snow)	Basic roof snow load factor Cb = 0.80	Building Category = Category 2	
TCLL 0.0 Kd = 1.00 (Live)	Wind exposure Cw = 1.00	End Zone = No	
TCDL 10.0 Kd = 1.15 (Wind)	Slope factor Cs = From Geometry	Apply to left end vertical = Yes	
BCLL 10.0	Slippery roof factor = No	Apply to right end vertical = Yes	
BCDL 10.0	Accumulation factor Ca = 1.0 Case I, Ca = 1.25 Case II		
Spacing: 2-00-00 oc Plies: 1			

Material Summary

TC	2x4	SPF	#2
BC	2x4	SPF	#2
Webs	2x4	SPF	#2

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.26
Max CSI in BC PANEL	1 - 4	0.07
Max CSI in Web	5 - 2	0.03

...	Mem...	Ten	Comp	.CSI.
TC	1- 2	100	239	0.26
	2- 3	100	239	0.26
BC	1- 4	111	37	0.07
	3- 6	111	37	0.07
	4- 5	111	37	0.04
	5- 6	111	37	0.04
Web	2- 5	11	353	0.03

Maximum Factored Reaction Summary

-----Reaction Summary(Lbs)-----						
Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat
1	1-02-10	625	64	9-07-12		
5	4-11-06	440	0	9-07-12		
3	8-08-02	625	64	9-07-12		
Max Horiz = -31 / +31 at Joint 5						
Reactions not shown: down < 400 and up < 150						
---- Reaction Summary (plf) ----						
Jnt-Jnt	React	-Up-	--Width-			
1- 3	20	0	9-07-12	(reduced)		

Unfactored Reaction Summary

Jnt	Type	Snow	Live	Wind	Dead
	Pinned (Wall)	758	192	-257	385

Loads Summary

See Loadcase Report for load combinations and additional details.

Notes

Designed as per NBCC2015/TPIC2014 and applicable provincial codes ABC2019, BCBC2018, MBC2018, and OBC2012(Jan 2020 update). Valley Truss application only. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/360	L/999(-0.00)	1- 4
Vert DL	L/360	L/999(-0.00)	5- 6
Vert TL	L/180	L/999(-0.00)	1- 4
Horz TL	1.00in	(0.00) @Jt 1	

Bracing Data Summary

-----Bracing Data-----
Chords; continuous except where shown
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)

Plate Info

Plate	Grip	psi	Shear	pli	Tens.Pli
AS	Max	Min	@0	@45	@90
20G	341	249	753	637	589 1144 1130

Plate Placement Tol. = 0.250 inches
Plate Rotation Tol. = 5.0 Deg.
JSI Grip = (INPUT = 0.90)
JSI Metal = (INPUT = 1.00)