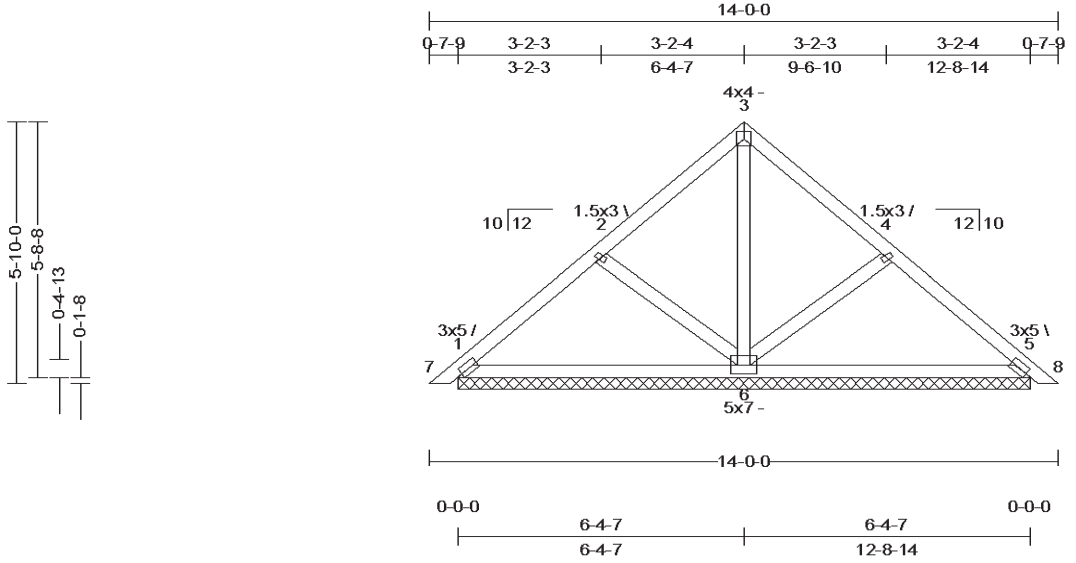
Truss: C1
JobName: GLT0524B-
Date: 07/19/19 14:43:18
Page: 1 of 1
Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
12-8-14	10 /12	24	0-7-9	0-7-9	0-0-0	0-0-0	1	24 in	49 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2009/	TC :	0.28 (1-2)	Vert TL:	0.04 in	L / 999	(5-6)	L / 240
TCDL :	10		TPI 1-2007	BC :	0.29 (6-1)	Vert LL:	0.02 in	L / 999	(5-6)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.17 (3-6)	Horz TL:	0 in			
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	152.875 in	N/A	918 lbs	·	-159 lbs	-134 lbs	-159 lbs	303 lbs
1	152.875 in	N/A	0 lbs	-431 lbs	-139 lbs	-94 lbs	-431 lbs	341 lbs
1	152.875 in	N/A	0 lbs	-431 lbs	-139 lbs	-94 lbs	-431 lbs	-341 lbs
1	152.875 in	N/A	1,091 lbs	·	·	·	·	699 lbs
1	152.875 in	N/A	1,091 lbs	·	·	·	·	-699 lbs

Material

TC: SPF#2 2 x 4
BC: SPF#2 2 x 4
Web: SPF#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (40 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 05 with the following user defined input: 40 psf Roof (GSL = 57 psf), Terrain C, Exposure (Ce = 1.0), Building Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 05 with the following user defined input: 90 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of a 14 psf live load computed in accordance with IBC 2009 assuming slope = 10 /12 and area supported = 28 ft^2, DOL = 115 %.
4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.282	-671 lbs								
	4-5	0.282	-671 lbs								
BC											
Web	2-6	0.140	-468 lbs	3-6	0.171	-341 lbs	4-6	0.140	-468 lbs		

Notes

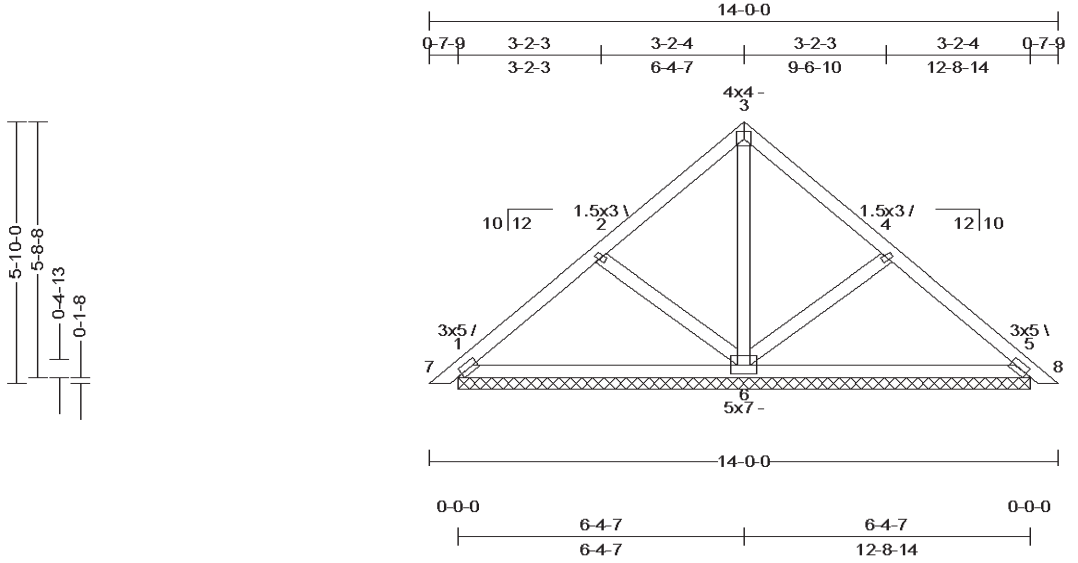
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 5, 1 may need to be considered.
5) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.6.338
Eagle Metal Products

Truss: C2
JobName: GLT0524B-
Date: 07/19/19 14:43:20

Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
12-8-14	10 /12	4	0-7-9	0-7-9	0-0-0	0-0-0	2	24 in	49 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2009/	TC :	0.14 (4-5)	Vert TL:	0.02 in	L / 999	(5-6)	L / 240
TCDL :	10		TPI 1-2007	BC :	0.14 (6-1)	Vert LL:	0.01 in	L / 999	(5-6)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.03 (4-6)	Horz TL:	0 in			
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	152.875 in	N/A	918 lbs	·	-159 lbs	-134 lbs	-159 lbs	303 lbs
1	152.875 in	N/A	0 lbs	-431 lbs	-139 lbs	-94 lbs	-431 lbs	341 lbs
1	152.875 in	N/A	0 lbs	-431 lbs	-139 lbs	-94 lbs	-431 lbs	-341 lbs
1	152.875 in	N/A	1,091 lbs	·	·	·	·	699 lbs
1	152.875 in	N/A	1,091 lbs	·	·	·	·	-699 lbs

Material

TC: SPF#2 2 x 4
BC: SPF#2 2 x 4
Web: SPF#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (40 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 05 with the following user defined input: 40 psf Roof (GSL = 57 psf), Terrain C, Exposure (Ce = 1.0), Building Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 05 with the following user defined input: 90 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of a 14 psf live load computed in accordance with IBC 2009 assuming slope = 10 /12 and area supported = 28 ft^2, DOL = 115 %.
4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.141	-335 lbs			
	4-5	0.141	-335 lbs			
BC						
Web						

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

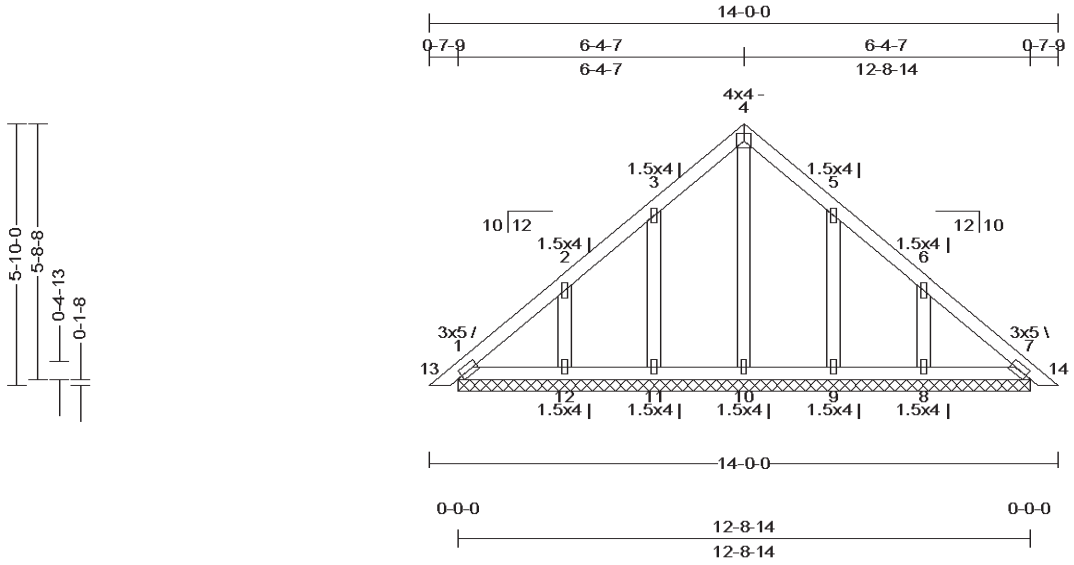
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
12-8-14	10 /12	4	0-7-9	0-7-9	0-0-0	0-0-0	2	24 in	49 lbs

4) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 5, 1 may need to be considered.
5) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 0.131"x3" Nails TC - 1 row @ 12 in oc, BC - 1 row @ 12 in oc , Webs - 1 row @ 12 in oc.
6) When applied loads are on one side of girder, do n ot flip girder during girder connector installation , install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
7) Lateral bracing shall be attached to each ply.
8) All fasteners minimum 2-1/2" long, unless otherwise noted.
9) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: CG1
JobName: GLT0524B-
Date: 07/19/19 14:43:23

Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
12-8-14	10 /12	2	0-7-9	0-7-9	0-0-0	0-0-0	2	24 in	52 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2009/	TC :	0.06 (1-2)	Vert TL:	0 in	L / 999	7	L / 240
TCDL :	10		TPI 1-2007	BC :	0.01 (8-9)	Vert LL:	0 in	L / 999	7	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.03 (5-9)	Horz TL:	0 in			
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	437 lbs	231 plf	-86 lbs	-96 lbs	-83 lbs	-96 lbs	-198 lbs

Material

TC: SPF#2 2 x 4
BC: SPF#2 2 x 4
Web: SPF#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (40 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 05 with the following user defined input: 40 psf Roof (GSL = 57 psf), Terrain C, Exposure (Ce = 1.0), Building Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 05 with the following user defined input: 90 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of a 14 psf live load computed in accordance with IBC 2009 assuming slope = 10 /12 and area supported = 28 ft^2, DOL = 115 %.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC		
BC		
Web		

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20gaplates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).

Truss: CG1
JobName: GLT0524B-
Date: 07/19/19 14:43:23

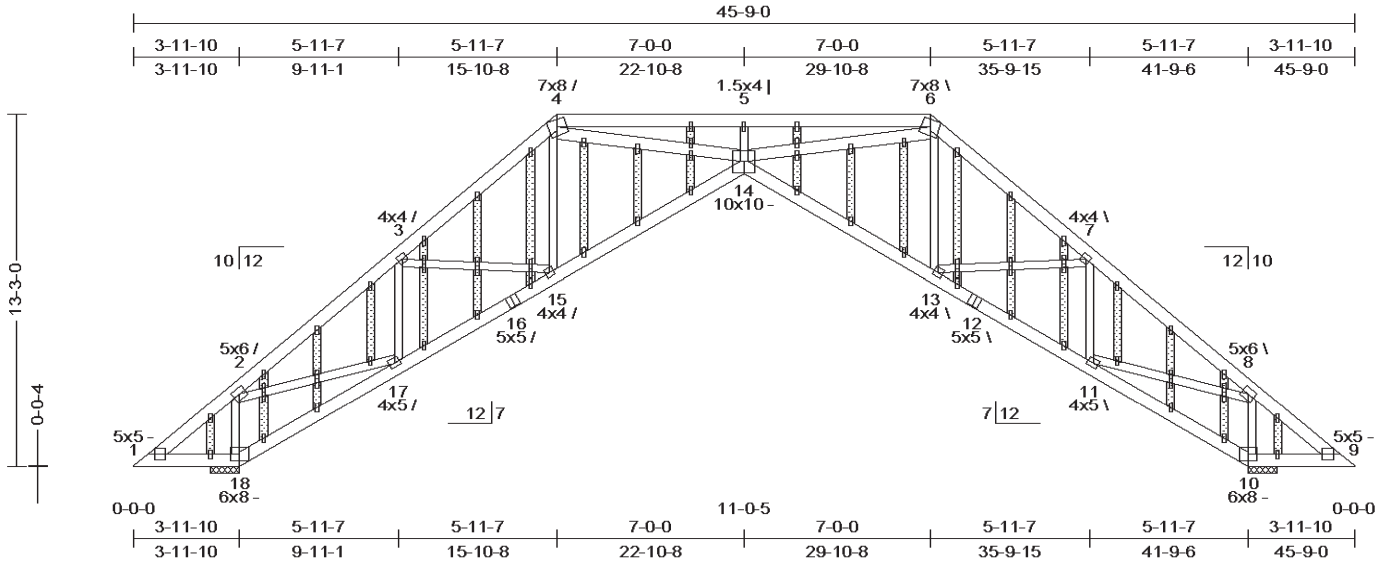
Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
12-8-14	10 /12	2	0-7-9	0-7-9	0-0-0	0-0-0	2	24 in	52 lbs

7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 1 may need to be considered.
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 0.131"x3" Nails TC - 1 row @ 12 in oc, BC - 1 row @ 12 in oc , Webs - 1 row @ 12 in oc.
9) When applied loads are on one side of girder, do n ot flip girder during girder connector installation , install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
10) Lateral bracing shall be attached to each ply.
11) All fasteners minimum 2-1/2" long, unless otherwise noted.
12) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
13) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: G1
JobName: GLT0524B
Date: 07/19/19 14:43:26
Page: 1 of 2
Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
45-9-0	10 /12	2	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	347 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2009/	TC :	0.67 (4-5)	Vert TL:	0.86 in	L / 528	14	L / 240
TCDL :	10		TPI 1-2007	BC :	0.83 (18-1)	Vert LL:	0.49 in	L / 921	14	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.42 (2-17)	Cant / OH TL:	0.14 in UP	2L / 401	(1-1)	2L / 240
BCDL :	10	Lumber D.O.L. :	115 %			Cant / OH LL:	0.08 in UP	2L / 704	(1-1)	2L / 240
						Horz TL:	1.05 in		10	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	13.125 in	N/A	6,457 lbs	·	-415 lbs	-295 lbs	-415 lbs	1,036 lbs
18	1	13.125 in	N/A	56 lbs	-3,254 lbs	-278 lbs	-69 lbs	-3,254 lbs	-1,036 lbs
10	1	13.125 in	N/A	6,446 lbs	·	-415 lbs	-294 lbs	-415 lbs	·
10	1	13.125 in	N/A	43 lbs	-3,244 lbs	-209 lbs	-70 lbs	-3,244 lbs	·

Material

TC: SPF#2 2 x 6
BC: SPF#2 2 x 6
Web: SPF#2 2 x 4 except:
SP 2400/2.0 2 x 6: 4-14, 6-14

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-8-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (40 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 05 with the following user defined input: 40 psf Roof (GSL = 57 psf), Terrain C, Exposure (Ce = 1.0), Building Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 05 with the following user defined input: 90 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of TC LL = 20 psf.
4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.453	843 lbs	(-35 lbs)	3-4	0.316	-2,254 lbs		5-6	0.672	-6,078 lbs		7-8	0.209	-1,607 lbs	
	2-3	0.209	-1,606 lbs		4-5	0.672	-6,078 lbs		6-7	0.316	-2,254 lbs		8-9	0.451	840 lbs	(-35 lbs)
BC	9-10	0.641	-516 lbs		11-13	0.397	1,375 lbs	(-54 lbs)	14-15	0.534	1,968 lbs	(-24 lbs)	17-18	0.333	-729 lbs	
	10-11	0.333	-726 lbs		13-14	0.534	1,968 lbs		15-17	0.397	1,374 lbs	(-79 lbs)	18-1	0.833	-518 lbs	
Web	2-18	0.207	-1,586 lbs		4-14	0.251	4,579 lbs	(-17 lbs)	7-11	0.110	-759 lbs					
	2-17	0.420	1,710 lbs	(-74 lbs)	5-14	0.041	-322 lbs		8-11	0.419	1,708 lbs	(-74 lbs)				
	3-17	0.110	-760 lbs		6-14	0.251	4,578 lbs	(-60 lbs)	8-10	0.206	-1,585 lbs					
	3-15	0.134	545 lbs		7-13	0.134	544 lbs	(-19 lbs)								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable webs placed at 24 " OC, U.N.O.
3) Attach structuralgable blocks with 1.5x4 20ga plates, U.N.O.
4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
6) Provide adequate drainage to prevent ponding.
7) ▨ Indicates non-structural members.

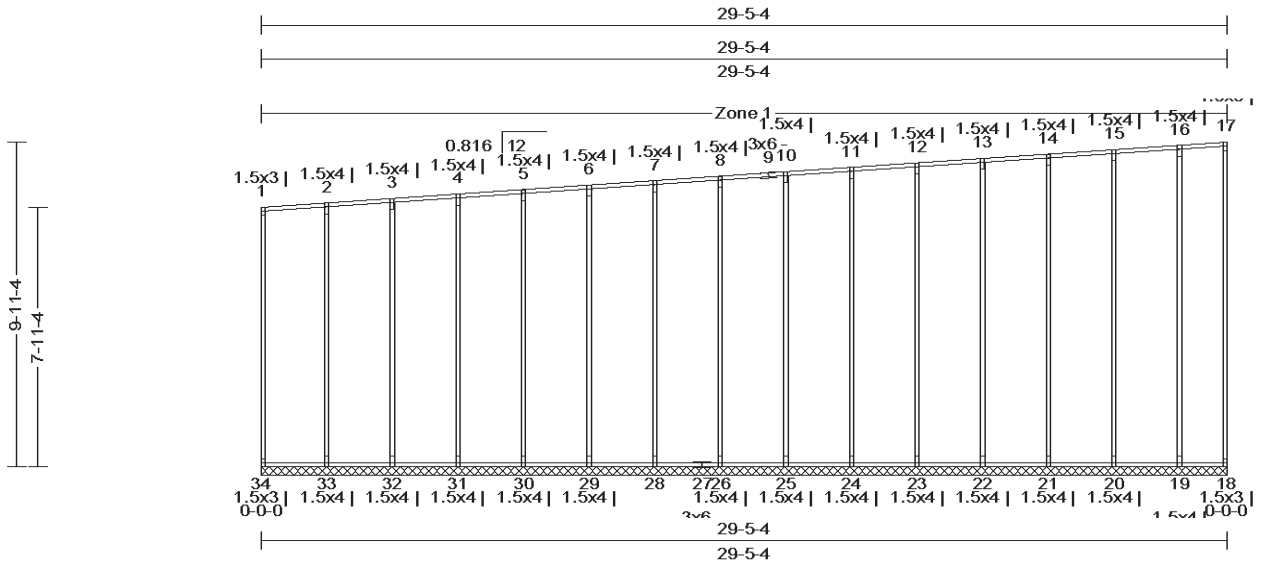
Truss: G1
JobName: GLT0524B-
Date: 07/19/19 14:43:26
Page: 2 of 2
Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
45-9-0	10 /12	2	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	347 lbs

8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 18, 10 may need to be considered.
9) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 0.131"x3" Nails TC - 2 staggered rows @ 12 in oc, BC - 2 staggered rows @ 12 in oc , Webs - 1 row @ 12 in oc.
10) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
11) Lateral bracing shall be attached to each ply.
12) All fasteners minimum 2-1/2" long, unless otherwise noted.
13) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
14) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: SFL1
JobName: GLT0524B-
Date: 07/19/19 14:43:28
Page: 1 of 1
Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	PLYS	SPACING	WGT/PLY
29-5-4	0.816 /12	4	0-0-0	0-0-0	1	Variable	275 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
TCLL : 40	Bldg Code :	IBC 2009/	TC : 0.00 (1-2)	Vert TL:	0 in	L / 999	18	L / 360
TCDL : 10		TPI 1-2007	BC : 0.00 (18-19)	Vert LL:	0 in	L / 999	18	L / 480
BCLL : 0	Rep Mbr :	Yes	Web : 0.00 (1-34)	Horz TL:	0 in			
BCDL : 10	Lumber D.O.L. :	100 %						

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	.	0 lbs	.	.	.	.	.	.

Material

TC: SYP #1 4 x 2
BC: SYP #1 4 x 2
Web: SYP #1 4 x 2

Loads

1) Unbalanced roof live loads have not been considered.

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC		
BC		
Web		

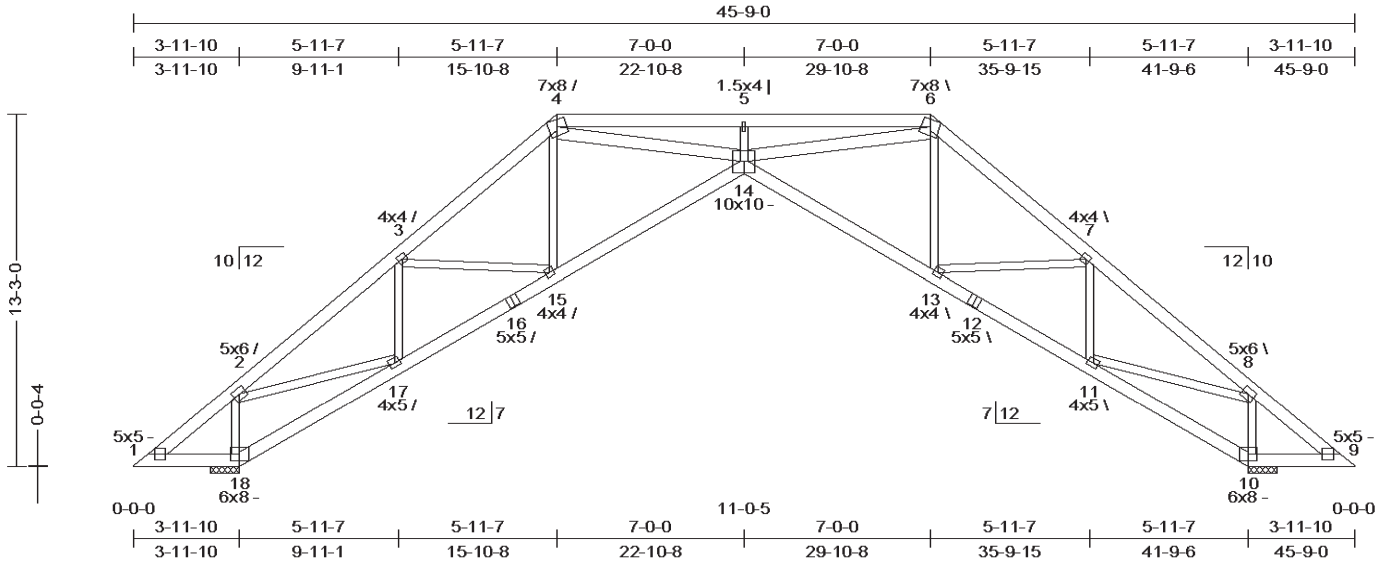
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) Gable requires continuous bottom chord bearing.
4) Gable webs placed at 24 " OC, U.N.O.
5) Attach gable webs with 1.5x4 20 gaplates, U.N.O.
6) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
7) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

Truss: T1
JobName: GLT0524B-
Date: 07/19/19 14:43:31

Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
45-9-0	10 /12	4	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	270 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2009/	TC :	0.67 (4-5)	Vert TL:	0.86 in	L / 528	14	L / 240
TCDL :	10		TPI 1-2007	BC :	0.83 (18-1)	Vert LL:	0.49 in	L / 921	14	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.42 (2-17)	Cant / OH TL:	0.14 in UP	2L / 401	(1-1)	2L / 240
BCDL :	10	Lumber D.O.L. :	115 %			Cant / OH LL:	0.08 in UP	2L / 704	(1-1)	2L / 240
						Horz TL:	1.05 in		10	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	13.125 in	N/A	6,457 lbs	·	-415 lbs	-295 lbs	-415 lbs	1,036 lbs
18	1	13.125 in	N/A	56 lbs	-3,254 lbs	-278 lbs	-69 lbs	-3,254 lbs	-1,036 lbs
10	1	13.125 in	N/A	6,446 lbs	·	-415 lbs	-294 lbs	-415 lbs	·
10	1	13.125 in	N/A	43 lbs	-3,244 lbs	-209 lbs	-70 lbs	-3,244 lbs	·

Material

TC: SPF#2 2 x 6
BC: SPF#2 2 x 6
Web: SPF#2 2 x 4 except:
SP 2400/2.0 2 x 6: 4-14, 6-14

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-8-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (40 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 05 with the following user defined input: 40 psf Roof (GSL = 57 psf), Terrain C, Exposure (Ce = 1.0), Building Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 05 with the following user defined input: 90 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of TC LL = 20 psf.
4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.453	843 lbs	(-35 lbs)	3-4	0.316	-2,254 lbs		5-6	0.672	-6,078 lbs		7-8	0.209	-1,607 lbs	
	2-3	0.209	-1,606 lbs		4-5	0.672	-6,078 lbs		6-7	0.316	-2,254 lbs		8-9	0.451	840 lbs	(-35 lbs)
BC	9-10	0.641	-516 lbs		11-13	0.397	1,375 lbs	(-54 lbs)	14-15	0.534	1,968 lbs	(-24 lbs)	17-18	0.333	-729 lbs	
	10-11	0.333	-726 lbs		13-14	0.534	1,968 lbs		15-17	0.397	1,374 lbs	(-79 lbs)	18-1	0.833	-518 lbs	
Web	2-18	0.207	-1,586 lbs		4-14	0.251	4,579 lbs	(-17 lbs)	7-11	0.110	-759 lbs					
	2-17	0.420	1,710 lbs	(-74 lbs)	5-14	0.041	-322 lbs		8-11	0.419	1,708 lbs	(-74 lbs)				
	3-17	0.110	-760 lbs		6-14	0.251	4,578 lbs	(-60 lbs)	8-10	0.206	-1,585 lbs					
	3-15	0.134	545 lbs		7-13	0.134	544 lbs	(-19 lbs)								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

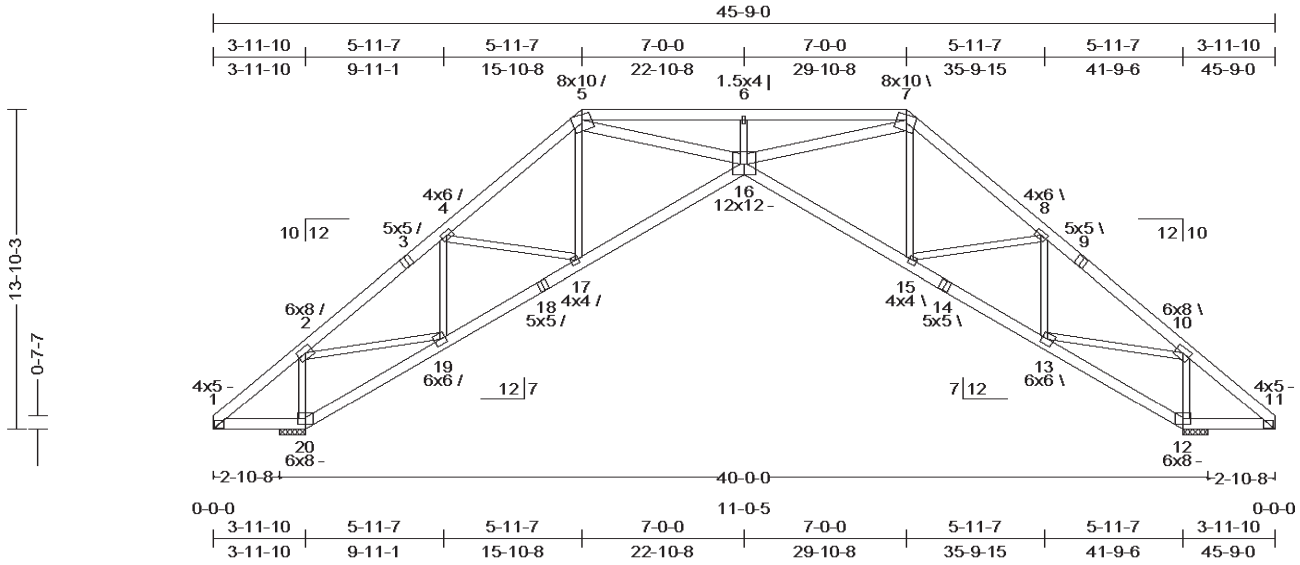
Truss: T1
JobName: GLT0524B-
Date: 07/19/19 14:43:31
Page: 2 of 2
Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
45-9-0	10 /12	4	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	270 lbs

5) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 18, 10 may need to be considered.
6) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 0.131"x3" Nails TC - 2 staggered rows @ 12 in oc, BC - 2 staggered rows @ 12 in oc , Webs - 1 row @ 12 in oc.
7) When applied loads are on one side of girder, do n ot flip girder during girder connector installation , install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
8) Lateral bracing shall be attached to each ply.
9) All fasteners minimum 2-1/2" long, unless otherwise noted.
10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: T2
JobName: GLT0524B
Date: 07/19/19 14:43:33
Page: 1 of 1
Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
45-9-0	10 /12	24	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	335 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2009/	TC :	0.59 (5-6)	Vert TL:	0.85 in	L / 535	16	L / 240
TCDL :	10		TPI 1-2007	BC :	0.38 (20-1)	Vert LL:	0.49 in	L / 932	16	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.70 (2-19)	Cant / OH TL:	0.15 in UP	2L / 449	(1-1)	2L / 240
BCDL :	10	Lumber D.O.L. :	115 %			Cant / OH LL:	0.09 in UP	2L / 791	(1-1)	2L / 240
						Horz TL:	1.06 in		12	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
20	1	13.125 in	N/A	4,785 lbs	·	-380 lbs	-284 lbs	-380 lbs	507 lbs
20	1	13.125 in	N/A	36 lbs	-1,582 lbs	-157 lbs	-80 lbs	-1,582 lbs	-507 lbs
12	1	13.125 in	N/A	4,781 lbs	·	-380 lbs	-283 lbs	-380 lbs	·
12	1	13.125 in	N/A	8 lbs	-1,579 lbs	-109 lbs	-81 lbs	-1,579 lbs	·

Material

TC: SP 2400/2.0 2 x 6
BC: SP 2400/2.0 2 x 6
Web: SPF #2 2 x 4 except:
SP 2400/2.0 2 x 6: 5-16, 7-16

Bracing

TC: Sheathed or Purlins at 2-9-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (40 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 05 with the following user defined input: 40 psf Roof (GSL = 57 psf), Terrain C, Exposure (Ce = 1.0), Building Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 05 with the following user defined input: 90 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of TC LL = 20 psf.
4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.246	899 lbs	(-68 lbs)	4-5	0.293	-4,320 lbs		6-7	0.595	-9,580 lbs		8-10	0.180	-3,198 lbs	
	2-4	0.180	-3,198 lbs		5-6	0.595	-9,580 lbs		7-8	0.293	-4,321 lbs		10-11	0.246	897 lbs	(-67 lbs)
BC	11-12	0.295	-506 lbs		13-15	0.265	2,733 lbs	(-55 lbs)	16-17	0.326	3,776 lbs	(-59 lbs)	19-20	0.225	-789 lbs	
	12-13	0.225	-788 lbs		15-16	0.326	3,777 lbs		17-19	0.265	2,733 lbs	(-158 lbs)	20-1	0.383	-507 lbs	
Web	2-20	0.564	-2,928 lbs		5-17	0.285	-444 lbs		7-15	0.285	-444 lbs		10-12	0.564	-2,927 lbs	
	2-19	0.700	2,853 lbs	(-109 lbs)	5-16	0.383	7,000 lbs	(-46 lbs)	8-15	0.235	957 lbs	(-35 lbs)				
	4-19	0.557	-1,483 lbs		6-16	0.103	-700 lbs		8-13	0.557	-1,483 lbs					
	4-17	0.235	957 lbs		7-16	0.383	7,000 lbs	(-129 lbs)	10-13	0.700	2,853 lbs	(-108 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 20, 12 may need to be considered.
6) Listed wind uplift reactions based on MWFRS & C&C loading.