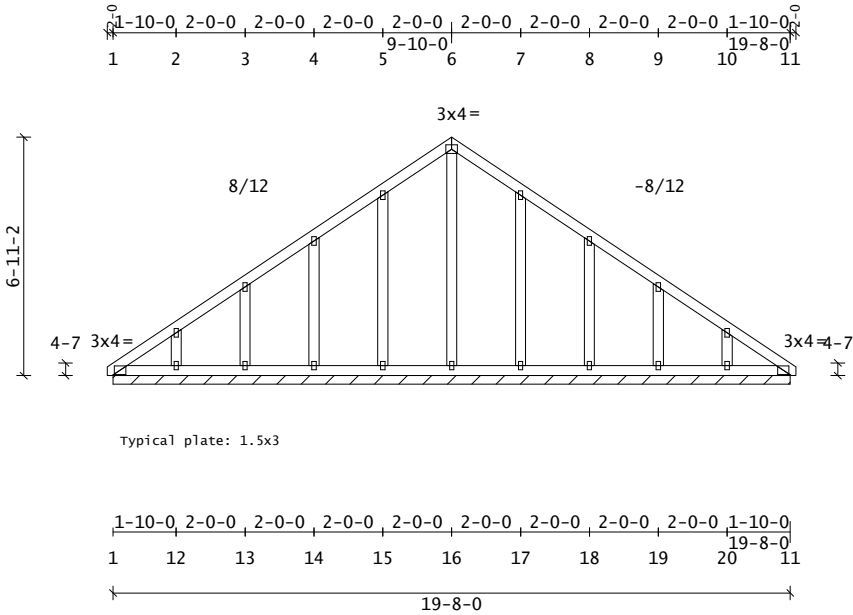


Customer: EZ SIPS Corporation

SID:  
TID: RB23075\_RP  
Date: 05 / 03 / 24  
Page: 1 of 1



Truss Weight = 86.7 lb

Code/Design: IBC-2021/TPI-2014  
PSF Live Dead Dur Factors  
TC 30.0 10.0 Live Wind Snow  
BC 0.0 10.0 Lum 1.25 1.60 1.15  
Total 50.0 Plt 1.25 1.60 1.15  
Spacing: 2-00-00 o.c. Plies: 1  
Repetitive Member Increase: Yes  
Green Lumber: No Wet Service: No  
Fab Tolerance: 20% Creep (Kcr) = 2.0  
OH Soffit Load: 2.0 psf

-----Snow Load Specs-----  
ASCE7-16 Ground Snow (Pg) = 30.0 psf  
Risk Cat: II Terrain Cat: C  
Roof Exposure: Sheltered  
Thermal Condition: All Others(1.0)  
Unobstructed Slippery Roof: No  
Low-Slope Minimums (P<sub>fmin</sub>): No  
Unbalanced Snow Loads: No  
Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----  
ASCE7-16 Wind Speed (V) = 115 mph  
Risk Cat: II Exposure Cat: C  
Bldg Dims: L = 49.0 ft B = 20.0 ft  
M.R.H(h) = 15.0 ft K<sub>zt</sub> = 1.0  
Bldg Enclosure: Enclosed  
Wind DL(psf): TC = 6.0 BC = 6.0  
End Vertical Exposed: L = Yes R = Yes  
Wind Uplift Reporting: ASCE7 MWFRS  
C&C End Zone: 3-00-00

-----Additional Design Checks-----  
10 psf Non-Concurrent BCLL: Yes  
20 psf BC Limited Storage: Yes  
200 lb BC Accessible Ceiling: Yes  
300 lb TC Maintenance Load: Yes  
2000 lb TC Safe Load: No  
Unbalanced TCCLL: Yes

Material Summary

TC	2x4	SPF	#1/#2
BC	2x4	SPF	#1/#2
Webs	2x4	SPF	#1/#2

Member Forces Summary

Max CSI in TC PANEL	4 - 5	0.08
Max CSI in BC PANEL	1 - 12	0.04
Max CSI in Web	16 - 6	0.12

...Mem...	Ten	Comp	.CSI.
TC 1- 6	97	78	0.08
6-11	97	78	0.08
BC 1-11	115	43	0.04
Web 2-12	106	214	0.02
3-13	89	224	0.03
4-14	89	222	0.06
5-15	83	226	0.11
6-16	19	158	0.12
7-17	83	226	0.11
8-18	89	222	0.06
9-19	89	224	0.03
10-20	106	214	0.02

Reaction Summary

Reactions not shown: down < 400 and up < 150  
---- Reaction Summary (plf) ----  
Jnt-Jnt React -Up- --Width-  
1- 11 101 3 19-08-00  
Max Horiz = -132 / +132 at Joint 16

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [9-10-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Notes

If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, BCSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.  
The maximum rake overhang length is 12.0".  
Plates designed for C<sub>q</sub> at 0.80 and Rotational Tolerance of 10.0 degrees.  
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.  
Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	19-20
Vert DL	L/120	L/999(-0.00)	12-13
Vert CR	L/180	L/999(-0.00)	12-13
Horz LL	0.75in	( 0.00)	@Jt 1
Horz CR	1.25in	( 0.00)	@Jt 1

Bracing Data Summary

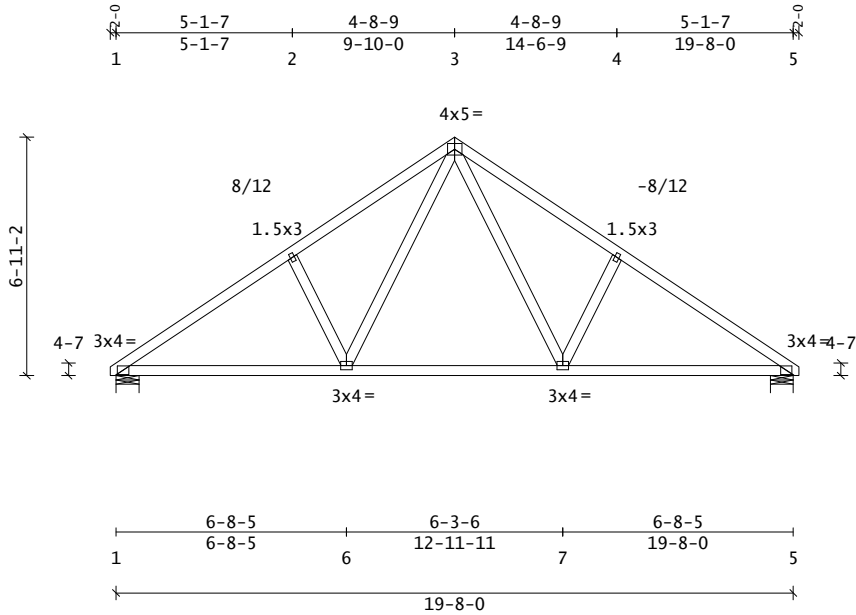
-----Bracing Data-----  
Chords; continuous except where shown  
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)

Customer: EZ SIPS Corporation

SID:  
TID: RB23075\_RP  
Date: 05 / 03 / 24  
Page: 1 of 1



Truss Weight = 73.8 lb

Code/Design: IBC-2021/TPI-2014  
PSF Live Dead Dur Factors  
TC 30.0 10.0 Live Wind Snow  
BC 0.0 10.0 Lum 1.25 1.60 1.15  
Total 50.0 Plt 1.25 1.60 1.15  
Spacing: 2-00-00 o.c. Plies: 1  
Repetitive Member Increase: Yes  
Green Lumber: No Wet Service: No  
Fab Tolerance: 20% Creep (Kcr) = 2.0  
OH Soffit Load: 2.0 psf

-----Snow Load Specs-----  
ASCE7-16 Ground Snow (Pg) = 30.0 psf  
Risk Cat: II Terrain Cat: C  
Roof Exposure: Sheltered  
Thermal Condition: All Others(1.0)  
Unobstructed Slippery Roof: No  
Low-Slope Minimums(Pfmin): No  
Unbalanced Snow Loads: No  
Rain Surcharge: No Ice Dam Chk: No

-----Wind Load Specs-----  
ASCE7-16 Wind Speed (V) = 115 mph  
Risk Cat: II Exposure Cat: C  
Bldg Dims: L = 49.0 ft B = 20.0 ft  
M.R.H(h) = 15.0 ft Kzt = 1.0  
Bldg Enclosure: Enclosed  
Wind DL(psf): TC = 6.0 BC = 6.0  
End Vertical Exposed: L = Yes R = Yes  
Wind Uplift Reporting: ASCE7 MWFRS  
C&C End Zone: 3-00-00

-----Additional Design Checks-----  
10 psf Non-Concurrent BCLL: Yes  
20 psf BC Limited Storage: Yes  
200 lb BC Accessible Ceiling: Yes  
300 lb TC Maintenance Load: Yes  
2000 lb TC Safe Load: No  
Unbalanced TCCLL: Yes

Material Summary

TC	2x4	SPF	#1/#2
BC	2x4	SPF	#1/#2
Webs	2x4	SPF	#1/#2

Member Forces Summary

Max CSI in TC PANEL	2 - 3	0.36
Max CSI in BC PANEL	1 - 6	0.48
Max CSI in Web	6 - 3	0.11

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	13	0	0.00
	1- 2	242	1374	0.34
	2- 3	300	1213	0.36
	3- 4	300	1213	0.36
	4- 5	242	1374	0.34
	5-OH	13	0	0.00
BC	1- 6	1061	126	0.48
	5- 7	1061	119	0.48
	6- 7	702	0	0.42
Web	2- 6	195	348	0.09
	3- 6	497	114	0.11
	3- 7	497	114	0.11
	4- 7	195	348	0.09

Reaction Summary

-----Reaction Summary(Lbs)-----							
Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat	PSI
1	02-07	996	38	08-00	01-09	SPF	425
5	19-05-09	996	38	08-00	01-09	SPF	425
Max Horiz = -132 / +132 at Joint 1							

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [9-10-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Lumber and plating have been applied symmetrically.

Deflection Summary

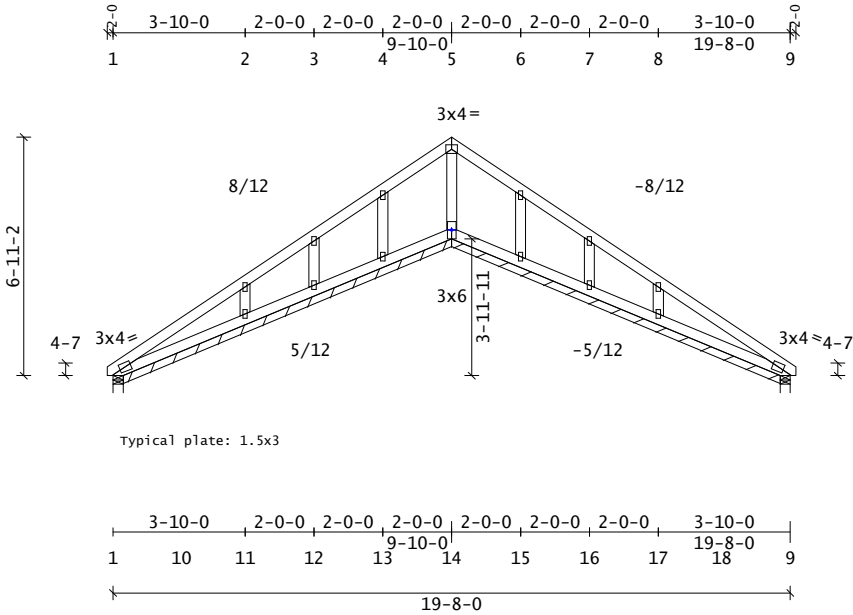
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.09)	6- 7
Vert DL	L/120	L/999(-0.05)	6- 7
Vert CR	L/180	L/999(-0.14)	6- 7
Horz LL	0.75in	( 0.02) @Jt 5	
Horz CR	1.25in	( 0.03) @Jt 5	

Bracing Data Summary

-----Bracing Data-----  
Chords; continuous except where shown  
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)



Truss Weight = 66.8 lb

Code/Design: IBC-2021/TPI-2014						-----Snow Load Specs-----						-----Wind Load Specs-----						-----Additional Design Checks-----											
PSF	Live	Dead		Dur	Factors	ASCE7-16	Ground	Snow(Pg)	=	30.0	psf	ASCE7-16	Wind	Speed(V)	=	115	mph	10	psf	Non-Concurrent	BCLL:	Yes							
TC	30.0	10.0		Live	Wind	Snow	Risk	Cat: II	Terrain	Cat: C		Risk	Cat: II	Exposure	Cat: C			20	psf	BC	Limited	Storage:	Yes						
BC	0.0	10.0	Lum	1.25	1.60	1.15	Roof Exposure: Sheltered					Bldg Dims: L = 49.0 ft B = 20.0 ft					200					lb	BC	Accessible	Ceiling:	Yes			
Total		50.0	Plt	1.25	1.60	1.15	Thermal Condition: All Others(1.0)					M.R.H(h) = 15.0 ft Kzt = 1.0					300					lb	TC	Maintenance	Load:	Yes			
Spacing: 2-00-00 o.c. Plies: 1						Unobstructed Slippery Roof: No						Bldg Enclosure: Enclosed						2000						lb	TC	Safe	Load:	No	
Repetitive Member Increase: Yes						Low-Slope Minimums(Pfmin): No						Wind DL(psf): TC = 6.0 BC = 6.0						Unbalanced						TCLL:	Yes				
Green Lumber: No Wet Service: No						Unbalanced Snow Loads: No						End Vertical Exposed: L = Yes R = Yes																	
Fab Tolerance: 20% Creep (Kcr) = 2.0						Rain Surcharge: No Ice Dam Chk: No						Wind Uplift Reporting: ASCE7 MWFRS																	
OH Soffit Load: 2.0 psf												C&C End Zone: 3-00-00																	

Material Summary

TC	2x4	SPF	#1/#2
BC	2x4	SPF	#1/#2
Webs	2x4	SPF	#1/#2

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.16
Max CSI in BC PANEL	1 - 10	0.14
Max CSI in Web	11 - 2	0.03

...Mem...	Ten	Comp	.CSI.
TC 1- 5	61	121	0.16
5- 9	61	121	0.16
BC 1-14	96	40	0.14
9-14	96	40	0.14
Web 2-11	147	268	0.03
3-12	72	218	0.02
4-13	89	231	0.03
5-14	46	168	0.02
6-15	89	231	0.03
7-16	72	218	0.02
8-17	147	268	0.03

Reaction Summary

-----Reaction Summary(Lbs)-----							
Jnt	--X-Loc-	React	-Up-	--Width-	-Reqd	-Mat	PSI
1	01-12	196	6	03-08	01-08	SPF	425
9	19-06-04	196	6	03-08	01-08	SPF	425
Reactions not shown: down < 400 and up < 150							
---- Reaction Summary (plf) ----							
Jnt-Jnt	React	-Up-	--Width-				
1- 14	96	0	9-06-08				
14- 9	80	12	9-06-08				
Max Horiz = -133 / +133 at Joint 14							

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [9-10-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Notes

If this truss is exposed to wind load perpendicular to the plane of the truss, gable studs must be braced according to the Construction Documents, ECSI-B3, or a gable stud bracing detail matching the design wind speed shown. Lateral bracing of the truss itself to resist out-of-plane wind load must be in accordance with the Construction Documents.  
The maximum rake overhang length is 12.0".  
Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees.  
Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints.  
Lumber and plating have been applied symmetrically.

Deflection Summary

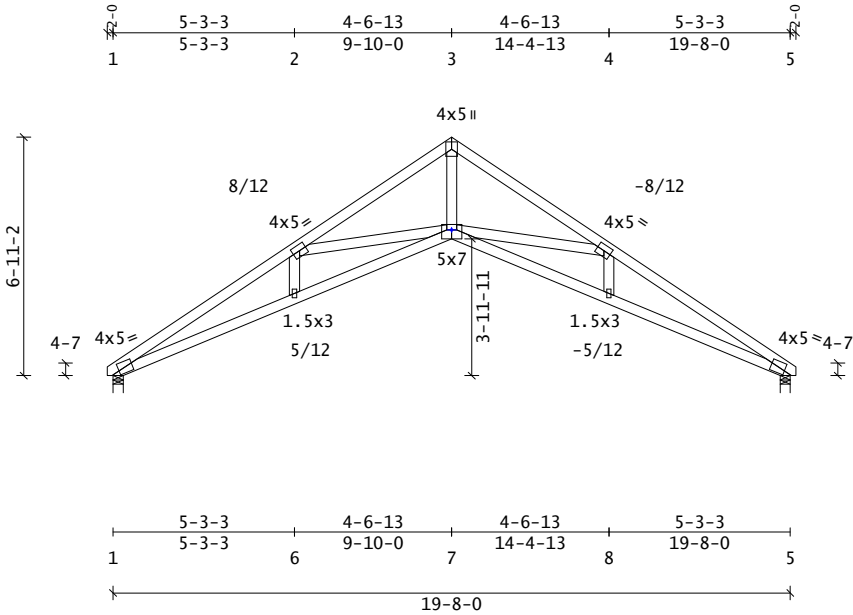
TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/999(-0.00)	18- 9
Vert DL	L/120	L/999(-0.00)	18- 9
Vert CR	L/180	L/999(-0.01)	18- 9
Horz LL	0.75in	( 0.00)	@Jt 9
Horz CR	1.25in	( 0.01)	@Jt 9

Bracing Data Summary

-----Bracing Data-----  
Chords; continuous except where shown  
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)  
Jnt14(0,-00-01)



Truss Weight = 72.7 lb

Code/Design: IBC-2021/TPI-2014		-----Snow Load Specs-----		-----Wind Load Specs-----		-----Additional Design Checks-----	
PSF	Live Dead	ASCE7-16 Ground Snow (Pg) = 30.0 psf		ASCE7-16 Wind Speed (V) = 115 mph		10 psf Non-Concurrent BCLL:	Yes
TC	30.0 10.0	Risk Cat: II Terrain Cat: C		Risk Cat: II Exposure Cat: C		20 psf BC Limited Storage:	Yes
BC	0.0 10.0	Roof Exposure: Sheltered		Bldg Dims: L = 49.0 ft B = 20.0 ft		200 lb BC Accessible Ceiling:	Yes
Total	50.0	Thermal Condition: All Others(1.0)		M.R.H(h) = 15.0 ft Kzt = 1.0		300 lb TC Maintenance Load:	Yes
Spacing:	2'-00'-00" o.c.	Unobstructed Slippery Roof: No		Bldg Enclosure: Enclosed		2000 lb TC Safe Load:	No
Repetitive Member Increase:	Yes	Low-Slope Minimums (Pfmin): No		Wind DL(psf): TC = 6.0 BC = 6.0		Unbalanced TCCLL:	Yes
Green Lumber:	No	Unbalanced Snow Loads: No		End Vertical Exposed: L = Yes R = Yes			
Fab Tolerance:	20% Creep (Kcr) = 2.0	Rain Surcharge: No Ice Dam Chk: No		Wind Uplift Reporting: ASCE7 MWFRS			
OH Soffit Load:	2.0 psf			C&C End Zone: 3'-00'-00"			

Material Summary

TC	2x4	SPF	#1/#2
BC	2x4	SPF	#1/#2
Webs	2x4	SPF	#1/#2

Member Forces Summary

Max CSI in TC PANEL	1 - 2	0.45
Max CSI in BC PANEL	1 - 6	0.75
Max CSI in Web	7 - 3	0.46

...	Mem...	Ten	Comp	.CSI.
TC	OH- 1	11	0	0.00
	1- 2	420	3079	0.45
	2- 3	208	2285	0.44
	3- 4	207	2285	0.44
	4- 5	403	3079	0.45
	5-OH	11	0	0.00
BC	1- 6	2668	301	0.75
	5- 8	2668	282	0.75
	6- 7	2692	302	0.73
	7- 8	2692	284	0.73
Web	2- 6	202	0	0.04
	2- 7	307	721	0.27
	3- 7	2042	102	0.46
	4- 7	310	721	0.27
	4- 8	202	0	0.04

Reaction Summary

-----Reaction Summary(Lbs)-----									
Jnt	--X-	Loc-	React	-Up-	--Width-	-Reqd	-Mat	PSI	
1		01-12	997	38	03-08	01-09	SPF	425	
5		19-06-04	997	38	03-08	01-09	SPF	425	
Max Horiz = -133 / +133 at Joint 1									

Loads Summary

This truss has been designed for the effects of an unbalanced top chord live load occurring at [9-10-00] using a 1.00 Full and 0.00 Reduced load factor.

See Loadcase Report for load combinations and additional details.

Notes

Plates designed for Cq at 0.80 and Rotational Tolerance of 10.0 degrees. Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Lumber and plating have been applied symmetrically.

Deflection Summary

TrussSpan	Limit	Actual(in)	Location
Vert LL	L/240	L/820(-0.28)	6- 7
Vert DL	L/120	L/999(-0.20)	6- 7
Vert CR	L/180	L/481(-0.48)	6- 7
Horz LL	0.75in	( 0.30) @Jt 5	
Horz CR	1.25in	( 0.50) @Jt 5	

Bracing Data Summary

-----Bracing Data-----  
Chords; continuous except where shown  
Web Bracing -- None

Plate offsets (X, Y):

(None unless indicated below)  
Jnt7(0,-00-09)

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "S# 18", which indicates a high tension 18 gauge plate.